

Development of Crash Reduction Factors

Peter W. Hovey, PhD
University of Dayton

Mashrur Chowdhury, PhD
Clemson University

Prepared for the
Ohio Department of Transportation
Office of Research and Development

State Job Number 14801(0)

September 2005



1. Report No. FHWA/OH-2005/12	2. Government Accession No.	3. Recipient's Catalog No.	
4. Title and subtitle Development of Crash Reduction Factors		5. Report Date September 2005	
		6. Performing Organization Code	
7. Author(s) Dr. Peter Hovey Dr. Mashrur (Ronnie) Chowdhury, P.E.		8. Performing Organization Report No.	
9. Performing Organization Name and Address University of Dayton 300 College Park Dr. Dayton, OH 45469		10. Work Unit No. (TRAVIS)	
		11. Contract or Grant No. 14801(0)	
12. Sponsoring Agency Name and Address Ohio Department of Transportation 1980 West Broad Street Columbus, Ohio 43223		13. Type of Report and Period Covered	
		14. Sponsoring Agency Code	
15. Supplementary Notes			
16. Abstract Crash reduction factors are used to identify and prioritize the most effective safety improvement measures, and prioritize and allocate available resources optimally for a highway safety improvement project. Simple before-and after analysis does account for the regression-to-the-mean bias. This research employs an Empirical Bayes (EB) methodology that overcomes the regression-to-the-mean property that is encountered in traditional before-and after analysis. Traffic, geometric and crash data for both the treatment and comparison sites were collected from Ohio in developing the crash reduction factors. Using data collected from Ohio, the EB methodology was applied in developing crash reduction factors for the following improvement categories:add a two-way left turn lane, install a median barrier, flatten slope and remove guardrail, remove or relocate a fixed object, flatten vertical curve, providing highway lighting and close median opening.			
17. Key Words Crash Reduction Factors, Empirical Bayes, Safety		18. Distribution Statement No restrictions. This document is available to the public through the National Technical Information Service, Springfield, Virginia 22161	
19. Security Classif. (of this report) Unclassified	20. Security Classif. (of this page) Unclassified	21. No. of Pages 86	22. Price
Form DOT F 1700.7 (8-72)		Reproduction of completed pages authorized	

Development of Crash Reduction Factors

Final Report

By:

Peter W. Hovey

Department of Mathematics

University of Dayton

and

Mashrur Chowdhury

Department of Civil Engineering

Clemson University

Acknowledgement: Prepared in cooperation with the Ohio Department of Transportation and the U.S. Department of Transportation, Federal Highway Administration.

Disclaimer: The contents of this report reflect the views of the authors who are responsible for the facts and the accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of the Ohio Department of Transportation or the Federal Highway Administration. This report does not constitute a standard, specification or regulation.

TABLE OF CONTENTS

1. Introduction.....	1
2. Literature Review on CRFs	2
2.1 Upgrade pavement markings--general	2
2.2 Install raised pavement markers, locations & sections	2
2.3 Install left turn bay & extra pavement without signal.....	3
2.4 Upgrade existing flasher	3
2.5 Install flasher.....	3
2.6 Upgrade existing signal - general	3
2.7 Install traffic signal - all types	4
2.8 Add left turn phase with new left turn lane.....	4
2.9 Add left turn phase with existing left turn lane.....	4
2.10 Add left turn phase without left turn lane	4
2.11 Install rumble strips	4
2.12 Upgrading Post-mounted signals to Mast-mounted.....	5
2.13 Upgrading Bidirectional Median Crossovers to Directional	5
2.14 Upgrading Intersections into Roundabouts.....	5
2.15 Seasonally Changing Speed Limits	6
3. Methodology	8
3.1 Statistical Considerations for Evaluating Crash Reduction Factors	8
3.2 Empirical Bayes Estimation.....	8
3.3 Multivariate Modeling	10
4. Data Collection	14
5. Modeling & Analysis.....	15
5.1 Add Two-way Left Turn Lane	15
5.2 Install Median Barrier	16
5.3 Flatten Slope, Remove Guardrail.....	17
5.4 Remove/Relocate Fixed Object	18
5.5 Flatten Vertical Curve.....	18
5.6 Provide Interchange Lighting	19
5.7 Close Median Opening	21
5.8 Computed CRFs.....	23

6. Conclusions and Recommendations.....	24
7. References.....	25
APPENDIX A - C++ Program for Data Extraction.....	A-1
APPENDIX B – EB Modeling Steps: Add Two-Way Left Turn Lanes.....	B-1
APPENDIX C – EB Modeling Steps: Install Median Barriers	C-1
APPENDIX D – EB Modeling Steps: Flatten Slope, Remove Guardrail.....	D-1
APPENDIX E – EB Modeling Steps: Remove/Relocate Fixed Objects	E-1
APPENDIX F – EB Modeling Steps: Flatten Vertical Curve	F-1
APPENDIX G – EB Modeling Steps: Provide Interchange Lighting	G-1
APPENDIX H – Summary of Selected Studies.....	H-1

LIST OF ABBREVIATIONS

CRF	Crash Reduction Factor
EB	Empirical Bayes
μ_λ	Mean of crash rates
σ_λ^2	Variance of crash rates
μ_{NB}	Mean of the negative binomial distribution
σ_{NB}^2	Variance of the negative binomial distribution
K	Site count

1. Introduction

Crash reduction factors (CRFs) represent the proportion of crashes that are expected to be eliminated from a site as a result of receiving a specific spot improvement(s). CRFs are used to identify and prioritize the most effective safety improvement measures and prioritize and allocate available resources optimally for a highway safety project. Crash reduction factors, which are based on previous research and field studies of before- and after crash statistics, need to be revised and updated periodically to reflect the most current knowledge regarding the effectiveness of various highway improvement measures. The validity of the CRFs is a key factor in maintaining project prioritization and resource allocation tasks in an optimal and orderly manner. Crash reduction factors have not been updated in Ohio since the early 1980s. It is important to verify and update these CRFs periodically to ensure their accuracy.

The primary objective of this research was to develop CRFs for seven improvement categories that were ranked by ODOT as most important. The researchers also performed a literature review to identify additional CRFs, which were developed from previous experimental studies, for various improvement categories.

The following seven improvement categories were selected by ODOT for model development and analysis for computing the CRFs.

- Add two-way left turn lane
The installation of a median lane that provides access to developments on both sides of the corridor.
- Install median barriers
The installation of new concrete barriers in the median.
- Remove/relocate fixed object
The removal of an object that is adjacent to the roadway, or the relocation of such an object to a safe distance from the roadway. These objects shall include utility poles, trees, guardrails, sign supports, and fire hydrants.
- Flatten slope, remove guardrail
Clear-zone grading or roadside grading is used to eliminate the need for barrier (guardrail) protection of a fixed object and slope protection. The guardrail is also removed in this improvement.
- Flatten vertical curve
The vertical realignment of the roadway such that a vertical curve gradient is reduced.
- Provide interchange lighting
The installation of lighting along the roadway to provide illumination. This study only evaluated interchange lighting because of data availability.

- Close median opening

The elimination of access through a median by construction of a raised median or the use of barricades.

2. Literature Review on CRFs

A literature review of published reports from various departments of transportation (DOTs) and transportation research organizations were performed. The CRFs reported below were developed based on experimental studies. In addition, several reports, such as the Kentucky Transportation Cabinet report on CRF (1996) included CRFs based on surveys or without providing any details on the process of development. We have not included any CRFs where we could not identify any statistical process on its development.

2.1 Upgrade pavement markings--general

Research conducted in the state of Indiana by Al-Masaeid and Sinha found a crash reduction factor of 0.13 for upgrading pavement markings. In the *Analysis of Accident Reduction Potentials of Pavement Markings* paper published in the Journal of Transportation Engineers, accident data from 2 years before and after was considered and computed the accident reduction factor by using the Bayesian approach to eliminate the regression-to-the-mean effect to account for biases. 7 sites where pavement markings were either installed or upgraded were studied.

2.2 Install raised pavement markers, locations & sections

Research conducted in the state of Indiana for the Indiana Department of Transportation found a crash reduction factor of 0.04 for the installation of raised pavement markers. In the *Accident Reduction Factors for Indiana* study by Purdue University, accident data from 3 years before and after was considered and before computing the accident reduction factor using regression-to-the-mean analysis, the accidents were adjusted using the traffic volumes rather than statewide growth factors to account for biases. 61 sites where raised pavement markers were installed were studied.

2.3 Install left turn bay & extra pavement without signal

Research conducted in the state of Iowa for the Iowa Department of Transportation found a crash reduction factor of 0.12 for the installation of a left turn bay and extra pavement without a signal. In the *Effectiveness of Roadway Safety Improvements* study by Iowa State University, accident data from 3 years before and after was considered and the empirical Bayesian Method was used to remove biases. 8 sites where a left turn bay and extra pavement was installed without a signal were studied.

2.4 Upgrade existing flasher

Research conducted in the state of Indiana for the Indiana Department of Transportation found a crash reduction factor of 0.09 for upgrading an existing flasher. In the *Accident Reduction Factors For Indiana* study by Purdue University, accident data from 3 years before and after was considered and before computing the accident reduction factor using regression-to-the-mean analysis, the accidents were adjusted using the traffic volumes rather than statewide growth factors to account for biases. 1 site where a flasher was upgraded was studied.

2.5 Install flasher

Research conducted in the state of Indiana for the Indiana Department of Transportation found a crash reduction factor of 0.07 for installing a flasher. In the *Accident Reduction Factors For Indiana* study by Purdue University, accident data from 3 years before and after was considered and before computing the accident reduction factor using regression-to-the-mean analysis, the accidents were adjusted using the traffic volumes rather than statewide growth factors to account for biases. 8 sites where flashers were installed were studied.

2.6 Upgrade existing signal - general

Research conducted in the state of Indiana for the Indiana Department of Transportation found a crash reduction factor of 0.11 for upgrading an existing signal. In the *Accident Reduction Factors For Indiana* study by Purdue University, accident data from 3 years before and after was considered and before computing the accident reduction factor using regression-to-the-mean analysis, the accidents were adjusted using the traffic volumes rather than statewide growth factors to account for biases. 110 sites where signals were upgraded were studied.

2.7 Install traffic signal - all types

Research conducted in the state of Iowa for the Iowa Department of Transportation found a crash reduction factor of 0.27 for the installation of all types of traffic signals. In the *Effectiveness of Roadway Safety Improvements* study by Iowa State University, accident data from 3 years before and after was considered and the empirical Bayesian Method was used to remove biases. 16 sites where signals were installed were studied.

2.8 Add left turn phase with new left turn lane

Research conducted in the state of Iowa for the Iowa Department of Transportation found a crash reduction factor of 0.20 when a left turn phase is added with a new left turn lane. In the *Effectiveness of Roadway Safety Improvements* study by Iowa State University, accident data from 3 years before and after was considered and the empirical Bayesian Method was used to remove biases. 11 sites where left turn phases were included with new left turn lanes were studied.

2.9 Add left turn phase with existing left turn lane

Research conducted in the state of Iowa for the Iowa Department of Transportation found a crash reduction factor of 0.36 when a left turn phase is added to an existing left turn lane. In the *Effectiveness of Roadway Safety Improvements* study by Iowa State University, accident data from 3 years before and after was considered and the empirical Bayesian Method was used to remove biases. 7 sites where left turn phases were added to with an existing left turn lane were studied.

2.10 Add left turn phase without left turn lane

Research conducted in the state of Iowa for the Iowa Department of Transportation found a crash reduction factor of 0.58 when a left turn phase is added without adding a left turn lane. In the *Effectiveness of Roadway Safety Improvements* study by Iowa State University, accident data from 3 years before and after was considered and the empirical Bayesian Method was used to remove biases. 4 sites where left turn phases were added without a left turn lane were studied.

2.11 Install rumble strips

Research conducted in the state of California for Caltrans found a crash reduction factor of 0.19 when rumble strips were installed along a stretch of roadway. In *The Evaluation of Accident Reduction Factors On California State Highways* study by California State University, accident data from 3 years before and after was considered and the empirical Bayesian Method was used to remove biases. 2 treatment sites and 3 reference sites were included in this study.

2.12 Upgrading Post-mounted signals to Mast-mounted

Research conducted in Kansas City, Missouri by the Public Works Department found a crash reduction factor of 0.25 for upgrading signal head mounting. In the *Impacts of Mast-Mounted Signal Heads on Accident Reduction* paper published in the Institute of Transportation Engineers (ITE) Journal, accident data from 1 year before and after was applied. A conservative Chi-squared test was used to test the significance. This test was chosen to eliminate biases, due to the limited amount of data available. Six intersections were included in this study

2.13 Upgrading Bidirectional Median Crossovers to Directional

Research conducted in the state of Michigan by the Michigan Department of Transportation and Michigan State University found a crash reduction rate of 0.30 for upgrading median crossovers. In the *Effect on Crashes After Construction of Directional Median Crossovers* paper, published in the Transportation Research Record, ten years of crash data were used for each site. A paired t-test was used to determine statistical significance of the results. It is important to note that since traffic volume information was not available for all of the years of this study, crash frequency was compared rather than crash per million vehicle mile traveled. Eight intersections were included in this study.

2.14 Upgrading Intersections into Roundabouts

Research conducted in seven states for the Insurance Institute for Highway Safety found a crash reduction rate of 0.40 for upgrading intersections into roundabouts. This study was conducted also in conjunction with Ryerson Polytechnic University and University of Maine. In the paper *Safety Effect of Roundabout Conversions in the United States*, published in the Transportation Research Record, crash data for at least 15 months before and after was applied. An empirical Bayesian method was applied to remove biases.

This study observed 23 intersections, including rural and urban single lane roundabouts, and urban multilane roundabouts. These categories were not large enough to draw statistically significant conclusions.

2.15 Seasonally Changing Speed Limits

Research conducted in Finland by the Technical Research Center of Finland, found a crash reduction factor of 0.14 for seasonally changing speed limits. This study was conducted also in conjunction with the Ministry of Transport and Communications and the Finnish National Road Administration. In the paper, *Seasonally Changing Speed Limits*, published in the Transportation Research Record, crash data for one year before and after were studied. A 10 year follow-up study confirmed the initial results. The speed limits were lowered 10 km/h for 4 months during winter. A total of 4000 km of roadway were studied initially. This was broken into 294 sections. An additional 7000 km of roadway were added in the follow-up study period. Generalized linear modeling techniques and t-tests were used to determine significance.

2.16 Wet Pavement Crashes

A CALTRAN study reported a crash reduction factor of 30% for pavement grooving and 25% for open graded asphalt overlays. These reduction factors apply to wet pavement crashes only. This study employed Empirical Bayes and Frequentist methods to estimate the crash reduction factors. In estimating the crash reduction factor for pavement grooving, only one treatment site and 10 corresponding reference sites were used. The estimate of crash reduction factor for open graded asphalt overlays included 4 treatment sites and 29 reference sites.

2.17 Left Turn Priority Treatment at Intersections

A study conducted in Toronto, Canada evaluated two types of left turn priority treatments at intersections: flashing advanced green and left turn green arrow. These priority phasing were applied as a leading operation at one or more approaches during certain period of a day. This study evaluated the safety effectiveness of priority treatments at 35 intersections in the City of Toronto over the three year period 1997-1999 using empirical Bayes methodology. This study found a 16 percent reduction of left turn crashes with a standard error of 4% for implementing flashing advanced green at intersections and a 17 percent reduction in left turn crashes with a standard error of 3.2% for implementing a left turn green arrow signal indication.

2.18 Traffic Signal Backboards Conspicuity

This study evaluated the safety impacts of improved signal conspicuity with the improvements to the traffic signal backboards (Sayed et al., 2005). Improvements include addition of yellow micro-prismatic retroreflective sheeting along the outer edge in order to frame the signal head and make these more visible to the drivers. This study employed time series analysis and empirical Bayes methodology to evaluate the safety impacts. Safety evaluations were done on 17 signalized intersections. The study found that the visibility improvements to the traffic signal backboards reduced crashes about 15 percent.

2.19 Two-Way Left Turn Lane

Bowman et al., found that arterials with two-way left turn lanes had lower vehicular crash rates compared to those with raised curb medians and undivided cross section in the central business districts. In the central business district area, they found a 21 percent lower crash rate for the midblock median segments of arterials with two-way left lane compared to the median mid block segments of an undivided highway.

2.20 Directional Median Crossovers

This study evaluated safety impacts of replacing bidirectional crossovers with directional median crossovers on arterial streets. Eight arterial road segments were studied where these changes were made, which varied in length from 1.17 km to 8.91 km. This study evaluated 54 bidirectional crossovers that were replaced by directional crossovers on these eight segments. Analysis of data showed a 30 percent reduction in both total crashes and crashes involving at least one injured party.

3. Methodology

The following section presents the statistical methodology adopted for developing CRFs.

3.1 Statistical Considerations for Evaluating Crash Reduction Factors

Evaluating the impact of roadway improvements poses a unique statistical challenge. The traditional approach of designed experiments is to select a random sample of subjects, apply the treatment to a random subset of the subjects and compare the responses of the two groups. The assumption is made that subjects are generally similar and that the treatment is relatively inexpensive to apply. Neither of these assumptions can be made in regard to roadway improvements.

Road improvements are a major undertaking and cannot practically be subjected to random sampling. Improvements are generally made in reaction to perceived problems at specific sites, which generally means the site has experienced a high number of crashes. The site could be unusually dangerous, or it could have just randomly experienced an unusual number of crashes. If the high crash rate were part of the natural distribution of crashes, the rate should go down without any improvement, a phenomenon known as regression to the mean. Because of the way sites are selected for improvement, it is impossible to discern whether a drop in observed crash rates is due to the improvement or due to regression to the mean without relating the results to comparison sites that have not undergone the improvement.

Despite general geometric similarities such as number of lanes and the presence of an intersection, there are still many factors that can vary between similar roadway sites. Average daily traffic (ADT) will have a big impact on the number of crashes and varies considerably between sites. Additional factors such as shoulder width and the type of development will also affect the number of crashes. These factors must be taken into account when comparing control and treatment sites.

The impact of regression to the mean can be reduced through the use of Empirical Bayes (EB) estimation, which uses distribution of crashes at the comparison sites to adjust the observed crashes at the treatment sites. Removing the impact of site variations is accomplished by building a multivariate model that predicts the expected number of crashes as a function of various site traits and measurements. The methods used in this study are detailed below.

3.2 Empirical Bayes Estimation

Bayesian estimation provides a means for combining information in a sample with another source of information to provide estimates of parameters. In a typical application, the other source of information, referred to as a prior distribution for the parameter, is based on previous experience or expert opinion. Empirical Bayes estimation uses data from similar experiments to formulate the prior distribution.

In roadway safety studies, the parameter is the crash rate for a specific site. Since relatively few observations will be collected for one site, data from similar sites must be used to improve precision. Empirical Bayes estimation provides the means for incorporating the information about crash rates from similar sites and reducing the impact of regression to the mean.

The Empirical Bayes estimation in roadway safety analysis has been adopted by several researchers (Hauer, et al., 2002 and Harwood, et al. 2000). The underlying theory of the Empirical Bayes analysis is that the crash rate at a specific site comes from a distribution that can be estimated by collecting crash data from a number of similar sites. Empirical Bayes estimation combines information about this distribution with data collected from a treatment site to offset the impact of a temporary, random increase in crashes.

The negative binomial distribution has been shown to be a reasonable model for the variation in number of crashes from year to year or site to site. The negative binomial model can be derived as a mixture of Poisson random variables with different rates. The number of crashes for an individual site can be modeled as a Poisson random variable. Since specific characteristics vary from site to site, the crash rates for individual sites will vary. The total crashes for different sites form a mixture of Poisson random variables with different rates. The result is that crash counts have a negative binomial distribution if crash rates have a gamma distribution. Experience has shown the negative binomial model to be a reasonable fit to observed data on crash counts.

In a Poisson random variable, the variance is equal to the mean; however this is not the case for the negative binomial distribution. The variance for the negative binomial is larger than the mean. One parameterization of the negative binomial is to use the mean and the overdispersion parameter. The overdispersion parameter measures the increase in the variance with respect to a Poisson distribution.

There are two common definitions of the overdispersion parameter. In his discussions of Empirical Bayes estimation, Hauer defines the overdispersion as ϕ where variance = mean*(1 + mean/ ϕ). In the SAS system (SAS 8.2), which is used in this study to conduct the analysis, the overdispersion parameter is $k = 1/\phi$, so that variance = mean*(1 + k*mean). This difference was taken into account in the calculations performed in this study.

The basic formula for the Empirical Bayes estimate of the mean number of crashes for a site, based on the negative binomial model is:

$$\hat{\mu}_{EB} = \alpha\mu_{NB} + (1 - \alpha)K, \quad \text{with } \alpha = \frac{\mu_{\lambda}}{(\mu_{\lambda} + \sigma_{\lambda}^2)} = \frac{\mu_{NB}}{\sigma_{NB}^2}$$

where, μ_{λ} and σ_{λ}^2 are the mean and variance of crash rates and μ_{NB} and σ_{NB}^2 are the mean and variance of the negative binomial distribution and K is the site count. The precision of an estimate is measured by the variance of the estimate, designated by

$V(\hat{\theta})$, or, equivalently, by the standard error, which is the square root of the variance of the estimate. The variance of the EB estimate of the mean number of crashes is:

$V(\hat{\mu}_{EB}) = (1 - \alpha)\mu_{EB}$ and it is estimated by using α , and $\hat{\mu}_{EB}$ from the previous equation.

In this form, the Empirical Bayes estimate assumes that the negative binomial distribution would apply to a group of sites that are exactly the same as the site under study. Estimating the parameters of this negative binomial distribution is impractical since even the most similar sites would have differences in some traits such as average daily traffic. A multivariate modeling approach is used to account for these differences.

3.3 Multivariate Modeling

Standard multivariate modeling builds a model of the mean as a function of various traits. The traits could be categorical or numerical and the assumption is made that the modeled variable has a normal distribution with a constant variance. Obviously, the number of crashes is not a normal random variable and using the negative binomial model we see that the variance is a function of the mean. A specialized form of multivariate modeling called negative binomial regression must be used for crash count data.

The general form for the model in negative binomial regression is:

$$\mu = \exp(\beta_0 + \sum \beta_i X_i)$$

Where μ is the mean and the X_i 's are the traits that are used to predict the mean. The iteratively reweighted least squares algorithm is used to estimate the parameters. The SAS system was used to conduct the computations in this study.

The strategy used to select the best traits to use in the model was a forward selection stepwise procedure. In the first step, all the traits that were available were fit to the data individually. The trait that individually provides the most information about crashes is selected for the model. The measure of information used in this process is the p-value for the factor from the SAS analysis. On subsequent tests, remaining traits, or factors, are individually added to the model and the one with the lowest p-value is added to the model, unless none of the factors is significant at the 0.05 level of significance. The process stops when none of the remaining factors is significant at the 0.05 level of significance.

If the p-value for any previously entered variable rose above the 0.05 level of significance it was removed from the model, except for Average Daily Traffic. If ADT rose above the 0.05 level of significance, the most recently added variable is removed.

Another concern in the model building process is that this is an observational study, so that all levels of class variables may not be represented in the data, or class variables may be confounded with other variables in the model. When class variables were entered into the model in the stepwise procedure, they were evaluated to make sure that these

problems did not apply. For example, when the functional classification (FC) of the roadway was included in one of the steps for two way left turn lanes, it was noted that only one level of FC was different from the others. Upon examining the data it was noted that only one site was in that FC category. In this case FC is confounded with the single site, so FC was not used in the final model.

The traits that were available in this study were: *midpoint* of the collection period, the duration of the collection period, average daily traffic, the percentage of truck traffic, the number of lanes, section length, the road width, the shoulder width, the median width and four classification variables MCL, SYS_CL, ACS and FC. The descriptions and levels of the classification variables are given below.

Explanation of Classification Variables

A). MCL: Indicates whether road is inside or outside of incorporated areas.

- 1 - Rural
- 2 - Municipal (incorporated)
- 4 - Rural and Municipal (split)

B). SYS CL: System classification

- I - Interstate
- M - Major thoroughfare
- A - Auxiliary
- L - Local, State

C). ACS: Highway Access Type as Journalized.

- N - No Access Control
- L - Limited Access Control
- F - Full Access Control

D). FC: Functional classification

- 01 - Rural Interstate
- 02 - Rural Principal Arterial
- 06 - Rural Minor Arterial
- 07 - Rural Major Collector
- 08 - Rural Minor Collector
- 09 - Rural Local
- 11 - Urban Interstate
- 12 - Urban Freeway & Expressway

14 - Urban Principal Arterial

16 - Urban Minor Arterial

17 - Urban Collector

19 - Urban Local

The data used in building the model included all time periods for the comparison sites and the data for the treatment sites that were collected prior to construction. The resulting model was then used to calculate the negative binomial mean and variance corresponding to the treatment sites for both the before and after construction time periods. The crash data from the treatments sites during construction was not used. The negative binomial means and variances were used with the actual count data for each time period to calculate the Empirical Bayes estimates for the crash rate for that site in that time period.

The next step in the process of calculating crash reduction factors is to project what the crash rates for the treatment sites would have been if the treatment had not been applied. The projections are based on the assumption that crash rates for an individual site maintain the same proportion to the average crash rates for all sites across time.

The projections are calculated by picking a base year from the time periods before construction and normalizing the mean crash rates for all time periods to the mean crash rate for the base. The projections of crash rates for the post construction period are independent of the choice of the base year. The normalized mean crash rate for year y is symbolized by C_y and is calculated as $C_y = \lambda_y/\lambda_b$, where λ_y and λ_b are the predicted crash rates from the multivariate model for year y and the base year.

The base value for projecting the expected post treatment crash rate is the weighted average of the Empirical Bayes estimates of crash rates of all years prior to construction. The formulae for the estimate of the base rate and an estimate of the sampling variance are:

$$\hat{\lambda}_b = \frac{\sum_{before} \hat{\lambda}_{EB,y}}{\sum_{before} C_y} \text{ with } V(\hat{\lambda}_b) = \frac{\sum_{before} V(\hat{\lambda}_{EB,y})}{\left(\sum_{before} C_y\right)^2}$$

The projected crash rate for the treatment site in year z after construction is

$$\hat{\lambda}_z = C_z \hat{\lambda}_b \text{ with } V(\hat{\lambda}_z) = C_z^2 V(\hat{\lambda}_b).$$

The crash reduction factors are calculated by comparing the actual crash counts after construction with the projected crash rates as calculated above. The crash reduction factor is derived from the index of effectiveness which is symbolized by θ . The index of effectiveness is the crash rate for an improved site divided by the crash rate for an unimproved site. The maximum likelihood estimate (mle) of the index of effectiveness is

$$\hat{\theta} = \frac{\sum_{after} K_z}{\sum_{after} \hat{\lambda}_z}$$

Since the mle of θ is the ratio of random variables, there is an inherent bias, which is estimated by

$$\hat{b} = 1 + \frac{\sum_{after} \hat{\lambda}_z}{\sum_{after} V(\hat{\lambda}_z)}$$

so that an unbiased estimate of θ is given by

$$\hat{\theta}_u = \frac{\hat{\theta}}{\hat{b}} \text{ with } V(\hat{\theta}_u) = \left(\frac{\hat{\theta}}{\hat{b}} \right)^2 \left(\frac{1}{\sum_{after} K_z} + \frac{\sum_{after} V(\hat{\lambda}_z)}{\left(\sum_{after} \hat{\lambda}_z \right)^2} \right).$$

The estimate of the crash reduction factor is then $CRF = 100(1 - \hat{\theta}_u)$ with the standard error given by $100\sqrt{V(\hat{\theta}_u)}$. The standard error represents the maximum error that will occur about 68 percent of the time.

The standard error can also be used to evaluate whether or not the estimated CRF is statistically significantly larger than 0. The standard difference of the CRF from 0 is calculated by dividing the estimate of the CRF by its standard error. Generally, values of the standard difference that are less than 1.65 are not considered significant while larger values are considered to be statistically significant.

A more precise evaluation is determined by calculating the p-value associated with the calculated standard difference. The p-value is the probability that a standard normal random variable will exceed the calculated standard difference. P-values less than 0.05 are generally considered significant while larger values are considered to be insufficient evidence that the CRF is larger than 0.

The limited amount of data and the large variability in the data in this study resulted in very few of the CRF estimates being significantly larger than 0. This should not be interpreted as evidence that the improvements are ineffective. The statistical tests are set up to evaluate the strength of evidence that CRF's are larger than 0, not the strength of evidence that they are equal to or less than 0. Collecting more data would likely increase the number of CRF estimates that are significantly larger than 0.

These basic procedures were applied to seven improvement categories. The specific results for each improvement category are discussed separately in Section 4.

4. Data Collection

A treatment site is defined as a roadway project where particular improvements were implemented to enhance its safety. A particular treatment group is a compilation of roadway projects, where each site has received the same safety treatment. Each treatment group consisted of two to five treatment sites. The following lists the treatment groups considered for this study:

1. Add two-way left turn lane.
2. Install median barriers.
3. Remove/relocate fixed object.
4. Flatten slope, remove guardrail.
5. Flatten vertical curve.
6. Provide interchange lighting.
7. Close median opening.

For each treatment site, the following roadway and traffic characteristics were considered for analysis using SAS program:

1. Roadway Width (RW)
2. Shoulder Width (SW)
3. Median Width (MW)
4. Crash Data
5. Average Daily Traffic (ADT)
6. Average Daily Trucks
7. Section Length

In this study, crash counts were collected for the before- and after- period following treatment. Most of the improvements were carried out in the mid 1990s. Information on physical characteristics of road segment and crash counts were made available by ODOT.

The ODOT Office of the Technical Services published seven different Roadway Inventory Reports for the development and maintenance of a Linear Referencing System (LRS) for state, county, township and municipal road and street systems:

1. State System Basic Road Inventory (RI-06)
2. Listing of Local Roads – sorted by county (RI-34A)
3. NHS and PAS Mileage by Functional Class (RI-339)
4. Listing of Local Roads Sorted by Township (RI-34B)
5. Centerline Miles, Lane Miles and Vehicle Miles Traveled Report (State Highway System only) (RI-82B)

6. State Highway System Lane Miles (RI-367)
7. Roadway Description Inventory Report (DESTAPE) – sorted by county and district.

The State System Basic Road Inventory (RI-06) is a list of data field descriptions. It contains physical characteristics such as surface type, surface width, roadway width, system classification, median width, highway access type, number of driving lanes, functional classification and urban area code of the specified road segment. DESTAPE helps to establish treatment and comparison sites location by route type and log points. The difference between begin log and end log gives length of the roadway segment. The treated road segment was located with the help of log distance on a particular route.

Crash data for treatment sites before, during and after treatment for a particular road section was collected by using –

- Annual reports on highway safety improvement programs for the State of Ohio
- Highway safety evaluation report of improvements
- Interaction with districts

Crash data for comparison sites was provided by the ODOT central office through their central crash history database. The UD research team compiled the received data into the required format for analysis.

The date of start and the date of completion of work for the treatment site denoted the precise construction period. The month in which construction began and ended were considered whole months for the treatment period. For example, for work that began on September 15, 1997 and ended on December 16, 1997, the construction period was from September 1997 to December 1997. It was ensured that crashes that occurred during the construction period were not added while computing the crash data.

Compiled crash data was further classified by crash type and severity. A C++ program was written and used to extract crashes by month, year and by crash type and severity for the before, during and after treatment periods (see Appendix A).

5. Modeling & Analysis

This section presents the modeling and analysis for the seven improvement categories.

5.1 Add Two-way Left Turn Lane

The forward selection procedure resulted in the models:

$$\mu_{\text{Total}} = \exp(-7.3310 + \ln(Dy) + 0.9613 \cdot \ln(\text{ADT}))$$

By following a similar forward selection process, the final model selected for I/F crashes was:

$$\mu_{\text{I/F}} = \exp(-8.3417 + \ln(\text{Dy}) + 0.9335 \cdot \ln(\text{ADT})), \text{ or}$$

Where Dy is an offset value for the duration of the time period. The Dy term corrects for the fact that some of the time periods immediately after construction did not last a full year. The Wald statistics, which are a test for significance of the terms in the model, indicate a level of significance of less than 0.0001 for total crashes and 0.0004 for injury and fatality crashes. Both P-values are significantly less than 0.05 indicating a strong fit.

In the model building phase, the class variable FC appeared to provide a statistically significant contribution to modeling crashes. A detailed analysis of the impact of FC indicated that only FC level 7, rural major collector, was significantly different from the other levels of FC. However, only one of the comparison sites was a rural major collector, so there is no way to determine whether the significant difference is due to the functional class, or to the specific site, so FC was not included in the model. This is a common problem in observational studies because there is no way to control the balance of levels of class variables.

After the model was selected, SAS was used to generate predicted values for each treatment site during all time periods both before and after construction. The predicted values are the means of the negative binomial distribution, which are used with the observed numbers of crashes to compute the Empirical Bayes (EB) estimates of the number of crashes for each observation of a treatment site. The SAS output for the final selected models are given in Appendix B. The EB estimates are used in the calculation of Crash Reduction Factors (CRF), which are also shown in Appendix B.

The estimated CRF for total crashes after adding a two way left turn lane is 0.083 with a standard error of estimate of 0.157. The standard difference from 0 is 0.52 with a p-value of 0.299 indicating little evidence that the CRF for two way left turn lanes is significantly larger than 0. The estimated CRF for injury and fatality accidents is 0.199 with a standard error of estimate of 0.254. The standard difference from 0 for the CRF for injury and fatality accidents is 0.78 with a p-value of 0.22, which is slightly more significant than for total crashes but still not generally considered statistically significant.

5.2 Install Median Barrier

The forward selection procedure resulted in the models:

$$\mu_{\text{Total}} = \exp(-19.6388 + \text{Dy} + 1.9505 \cdot \ln(\text{ADT}))$$

and

$$\mu_{\text{I/F}} = \exp(-20.7886 + \text{Dy} + 2.0602 \cdot \ln(\text{ADT}) - 0.0005 \cdot \text{Midpoint})$$

Where Dy is an offset value for the duration of the time period. The Dy term corrects for the fact that some of the time periods immediately after construction did not last a full year. The Wald statistics, which are a test for significance of the terms in the model, indicate a level of significance for both total crashes and injury and fatality crashes. The P-values for total crashes and injury and fatality crashes are significantly less than 0.05 indicating a strong fit.

After the model was selected, SAS was used to generate predicted values for each treatment site during all time periods both before and after construction. The predicted values are the means of the negative binomial distribution, which are used with the observed numbers of crashes to compute the Empirical Bayes (EB) estimates of the number of crashes for each observation of a treatment site. The SAS output for the final selected models are given in Appendix C. The EB estimates are used in the calculation of Crash Reduction Factors (CRF), which are also shown in Appendix C.

The estimated CRF for total crashes after Installing Median Barrier is 0.863 with a standard error of estimate of 0.029. The standard difference from 0 is 29.5 and the associated p-value is less than 0.0001. The estimated CRF for injury and fatality accidents is 0.884 with a standard error of estimate of 0.052. For injury and fatality accidents, the standard difference from 0 is 16.9 and the associated p-value is less than 0.0001. For both cases, there is strong statistical evidence of a reduction in crashes based on the data available in this study.

5.3 Flatten Slope, Remove Guardrail

The forward selection procedure resulted in the model:

$$\mu_{\text{Total}} = \exp(-6.4369 + Dy + 0.5703 \cdot \ln \text{ADT} + 0.6545 \cdot \ln \text{Trcks})$$

Where Dy is an offset value for the duration of the time period. The Dy term corrects for the fact that some of the time periods immediately after construction did not last a full year. The Wald statistics, which are a test for significance of the terms in the model, indicate a level of significance for total crashes. P-value is significantly less than 0.05 indicating a strong fit. The model for injury/fatal severity type was could not be developed as no injury/fatal crashes were observed for the sites selected in before and after period of improvement. Consequently, CRFs for I/F crashes could not be estimated for this category of improvement.

After the model was selected for total crashes, SAS was used to generate predicted values for each treatment site during all time periods both before and after construction. The predicted values are the means of the negative binomial distribution, which are used with the observed numbers of crashes to compute the Empirical Bayes (EB) estimates of the number of crashes for each observation of a treatment site. The SAS output for the final selected model is given in Appendix D. The EB estimates are used in the calculation of Crash Reduction Factors (CRF), which are also shown in Appendix D.

The estimated CRF for total crashes after flattening slopes and removing guardrail is 0.424 with a standard error of estimate of 0.575. The standard difference from 0 is 0.74

with a P-value of 0.23. The CRF for injury and fatality crashes could not be computed because there were no injury or fatality crashes reported for either treatment site during the course of the study.

5.4 Remove/Relocate Fixed Object

The forward selection procedure resulted in the models:

$\mu_{\text{Total}} = \exp (1.4827 + Dy + FC + 0.1995*\ln\text{ADT} - 1.1324*\ln\text{SW} + \text{ACS} - 0.4754*\ln\text{Trcks})$, where the FC term is equal to: 2.1956 for FC = 6, -0.2449 for FC=7, -1.3426 for FC = 8, 0.5334 for FC = 14, and 0.0 for FC = 16 and the ACS term is equal to - 0.6847 for ACS=L and 0.0 for ACS=N.

and

$\mu_{\text{TI/F}} = \exp (-1.1020 + Dy + FC + 0.4051*\ln\text{ADT} - 1.4265*\ln\text{SW} + \text{ACS})$, where the FC term is equal to: 2.2433 for FC = 6, -0.1002 for FC=7, -0.8018 for FC = 8, 0.4796 for FC = 14, and 0.0 for FC = 16 and the ACS term is equal to -0.6406 for ACS = L and 0 for ACS = N.

Where Dy is an offset value for the duration of the time period. The Dy term corrects for the fact that some of the time periods immediately after construction did not last a full year. The Wald statistics, which are a test for significance of the terms in the model, indicate a level of significance for both total crashes and injury and fatality crashes. The P-values for total crashes and injury and fatality crashes are significantly less than 0.05 indicating a strong fit.

After the model was selected, SAS was used to generate predicted values for each treatment site during all time periods both before and after construction. The predicted values are the means of the negative binomial distribution, which are used with the observed numbers of crashes to compute the Empirical Bayes (EB) estimates of the number of crashes for each observation of a treatment site. The SAS output for the final selected models are given in Appendix E. The EB estimates are used in the calculation of Crash Reduction Factors (CRF), which are also shown in Appendix E.

The estimated CRF for total crashes after removing or relocating a fixed object is 0.382 with a standard error of estimate of 0.103. The standard difference from 0 is 3.01 and the associated p-value is 0.0001. The estimated CRF for injury and fatality accidents is 0.381 with a standard error of estimate of 0.134. For injury and fatality accidents, the standard difference from 0 is 2.84 and the associated p-value is 0.0022. For both cases, there is a statistically significant reduction in crashes based on the data available in this study.

5.5 Flatten Vertical Curve

The forward selection procedure resulted in the models:

$\mu_{\text{Total}} = \exp (-2.9451 + Dy + 0.5470*\ln\text{ADT} + 0.8849*\ln\text{SL} + \text{SYS_CL})$,

with the SYS_CL term equal to -0.82 for SYS_CL = A, -0.18 for SYS_CL = L and 0 for SYS_CL = M,

and

$$\mu_{TVF} = \exp (-4.3707 + Dy + 0.7448*\ln ADT - 1.4310*\ln Trcks)$$

Where Dy is an offset value for the duration of the time period. The Dy term corrects for the fact that some of the time periods immediately after construction did not last a full year. The Wald statistics, which are a test for significance of the terms in the model, indicate a level of significance of less than 0.0004 for total crashes and 0.0024 for injury and fatality crashes. Both P-values are significantly less than 0.05 indicating a strong fit.

After the model was selected, SAS was used to generate predicted values for each treatment site during all time periods both before and after construction. The predicted values are the means of the negative binomial distribution, which are used with the observed numbers of crashes to compute the Empirical Bayes (EB) estimates of the number of crashes for each observation of a treatment site. The SAS output for the final selected models are given in Appendix F. The EB estimates are used in the calculation of Crash Reduction Factors (CRF), which are also shown in Appendix F.

The estimated CRF for total crashes after flattening the vertical curvature in the roadway profile is 0.196 with a standard error of estimate of 0.191. The standard difference from 0 is 1.02 and the associated p-value is 0.1526. The estimated CRF for injury and fatality accidents is 0.512 with a standard error of estimate of 0.190. For injury and fatality accidents, the standard difference from 0 is 2.69 and the associated p-value is 0.0035. Estimated CRF for injury and fatality accidents appear to be statistically significant reductions, whereas for total crashes it is slightly significant, based on the available data in this study.

The estimated CRF's for both the total crashes and injury and fatality accidents were calculated after removing treatment site #2 from the data set. Treatment site #2, from mile markers 8.56 to 9.16 on State Route 310 in Licking County, Ohio showed unusual crash statistics compared to the rest of the data set. In the before construction period, over a 3 year duration, 18 total crashes (6 injury and fatality accidents) occurred. However, after flattening the vertical curve, over a 3 year 304 day duration, 39 total crashes (16 injury and fatality accidents) occurred. This represents a 170% increase in total crashes and a 209% increase in injury and fatality accidents. Because of this drastic increase, treatment site #2 was excluded from the calculations, however the crash reduction factors with treatment site #2 included in the calculations are also shown in Appendix F.

5.6 Provide Interchange Lighting

The forward selection procedure resulted in the models:

$$\mu_{Total} = \exp (1.9432+ Dy - 0.2212*\ln ADT + 0.0405*SW)$$

And

$$\mu_{TIF} = \exp(-2.1013 + Dy + 0.1531 * \ln ADT)$$

Where Dy is an offset value for the duration of the time period. The Dy term corrects for the fact that some of the time periods immediately after construction did not last a full year. The Wald statistics, which are a test for significance of the terms in the model, indicate a level of significance for total crashes and injury and fatality crashes.

After the model was selected, SAS was used to generate predicted values for each treatment site during all time periods both before and after construction. The predicted values are the means of the negative binomial distribution, which are used with the observed numbers of crashes to compute the Empirical Bayes (EB) estimates of the number of crashes for each observation of a treatment site. The SAS output for the final selected models are given in Appendix G. The EB estimates are used in the calculation of Crash Reduction Factors (CRF), which are also shown in Appendix G.

The estimated CRF for total crashes after providing highway lighting is 0.504 with a standard error of estimate of 0.166. The standard difference from 0 is 3.04 with a P value of 0.0012. The estimated CRF for injury and fatality crashes is 0.260 with a standard error of 0.381. The standard difference from 0 for the CRF for injury and fatality crashes is only 0.68 with a p-value of 0.25 indicating weak statistical significance for this CRF value.

An additional analysis was attempted using only the crashes from 6:00 PM to 6:00 AM since highway lighting should mainly affect night time crashes. There were an insufficient number of injury and fatality crashes to calculate the CRF and although there were sufficient total crashes to calculate the CRF, the result should be used cautiously because of the low number of crashes.

The model for night time crashes was built using night time traffic figures. Night time traffic figures were generated by multiplying ADT by standard percentages based on functional class of the roadway. The final model selected for predicting night time crashes was:

$$\mu_{Night} = \exp(6.6802 + Dy - 1.4183 * \ln NDT + 1.13 * \ln Trcks + 0.0804 * SW)$$

where $\ln NDT$ is the logarithm of the average daily night time traffic counts. Note that according to this model an increase in night time traffic will result in a decrease in the night time crash rate. It is not clear why night time crashes should decrease with increased traffic. This could be an artifact of the small number of total night time crashes, or perhaps more traffic helps keep night time drivers awake.

The CRF for night time crashes was calculated to be 0.941 with a standard error of 0.057. The standard difference from 0 for total night time crashes is 16.4 with a p-value that is essentially 0. This is extreme statistical significance and may be impacted by the few number of night time crashes considered in this study.

5.7 Close Median Opening

The Empirical Bayes estimation procedure broke down for the data collected for the close median opening improvement. A basic assumption of the EB method is that the comparison sites and the treatment sites form a homogeneous population so that the crash rates at the treatment sites can be viewed as a random variable whose distribution is given by that population. Although the comparison sites were selected based on similarity of geometric configurations, they do not form a homogeneous population with the treatment sites selected for this category. The treatment sites were in the vicinity of the Dayton Mall, while the comparison sites were in primarily residential areas.

Figure 1 shows plots of crash rates per year per thousand ADT versus date for both treatment sites and their associated comparison sites. The treatment sites are labeled T1-1 and T2-1. The comparison sites are labeled by their association with the treatment sites as Cm-n where m represents the treatment site it is associated with and n represents the specific comparison site within the m treatment group. There is clear visual evidence that the crash rates for the treatment sites are much larger than the crash rates for the comparison sites. This is a violation of the assumptions behind the EB method, so that method could not be applied to this improvement category.

The changes in crash rates for the comparison sites were also too different from the changes in crash rates for the treatment sites, so the before/after method with comparison sites could not be used either. The naïve before/after analysis, which uses only the data from the treatment sites, was used for analyzing the data for the close median opening improvement category. The results for this improvement category should be used with caution, because of the need to use the naïve before and after analysis. There should be some effort to identify a better set of comparison sites and redo the analysis with the Empirical Bayes procedure at some point in the future.

The time periods before and after the improvements were constructed were of different lengths, so the crash counts were converted to rates per year by dividing by the duration. There was roughly an 8 percent increase in ADT after the construction, so the before crash rate was multiplied by 1.08 to account for the expected increase in crashes due to increased ADT. The index of effectiveness and crash reduction factors were then calculated from the after crash rate and the projection of the before crash rate to the after period.

The formulae for these calculations are:

$$\hat{\lambda}_i = \frac{K_i}{D_i} \quad \text{and} \quad V(\hat{\lambda}_i) = \frac{K_i}{D_i^2}, \quad \text{where } i = b \text{ or } a \text{ for before or after}$$

$$\hat{\lambda}_P = \hat{\lambda}_b \frac{ADT_{after}}{ADT_{before}} \quad \text{and} \quad V(\hat{\lambda}_P) = V(\hat{\lambda}_b) \left(\frac{ADT_{after}}{ADT_{before}} \right)^2$$

$$\hat{\theta} = \frac{\hat{\lambda}_a / \hat{\lambda}_P}{1 + V(\hat{\lambda}_P) / \hat{\lambda}_P^2} \quad \text{and} \quad V(\hat{\theta}) = \hat{\theta}^2 \left(\frac{V(\hat{\lambda}_a)}{\hat{\lambda}_a^2} + \frac{V(\hat{\lambda}_P)}{\hat{\lambda}_P^2} \right) \left/ \left(1 + \frac{V(\hat{\lambda}_P)}{\hat{\lambda}_P^2} \right)^2 \right.$$

$$CRF = (1 - \hat{\theta}) \quad \text{and} \quad se(CRF) = \sqrt{V(\hat{\theta})}$$

The specific estimates for total crashes are

$$\hat{\theta} = 0.93825 \quad \text{with} \quad \sqrt{V(\hat{\theta})} = 0.11302$$

The CRF for total crashes is 0.062 with a standard error of 0.11 and for injury and fatality crashes -0.032 with a standard error of 0.225. The standard score for total crashes being greater than 0 is 0.55 with a p-value of 0.29. There was an apparent increase in the rate of injury and fatality crashes. The results indicate that this particular improvement is not significantly impacting safety both in terms of total crashes and injury and fatality crashes.

Looking at Figure 1 it can be seen that crashes increased for one treatment site and decreased for the other. The net effect is that there is no apparent change in crash rates based on this data.

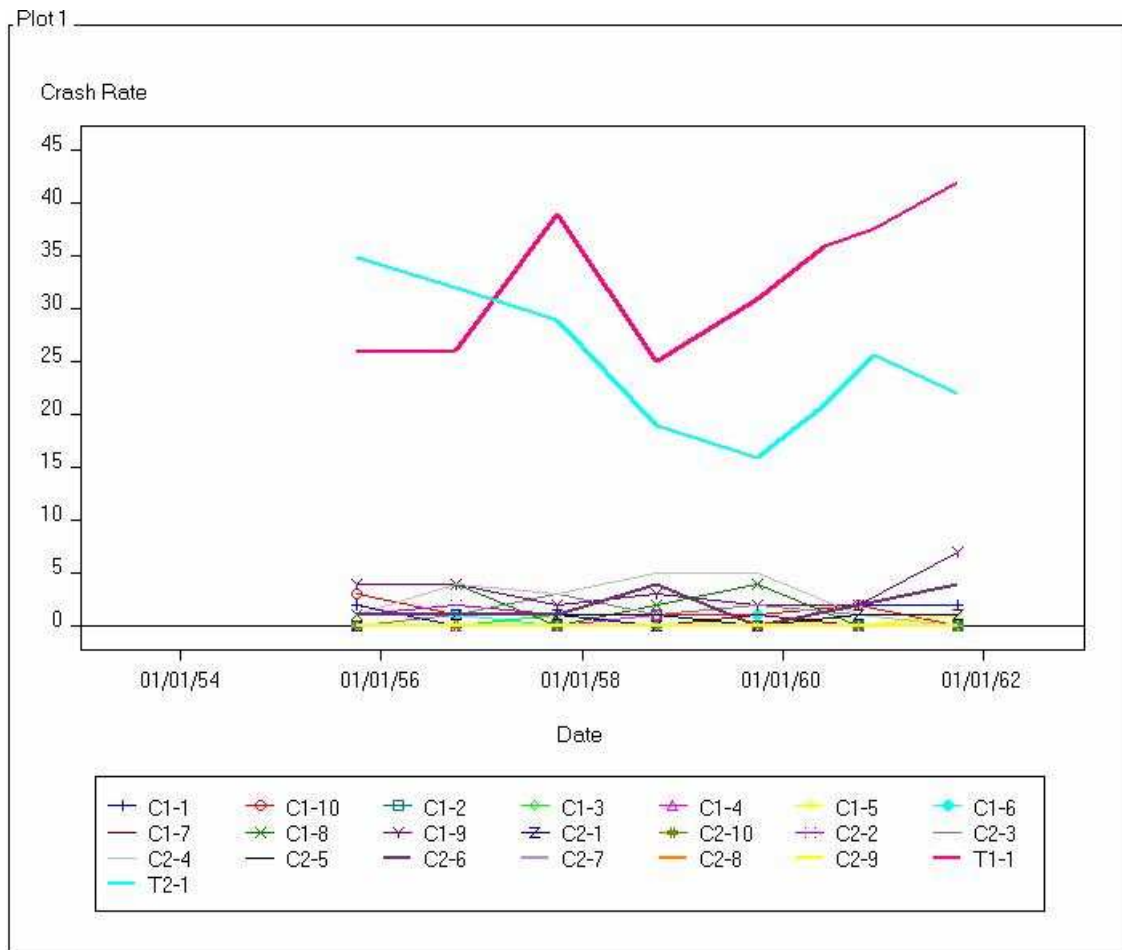


Figure 1. Time trends of crash rates for close median opening sites.

5.8 Computed CRFs

Table 1 provides a list of Crash Reduction Factors (CRFs) for both the total crashes and I/F crashes. These are experimental results, as these values were estimated based on field data.

Table 1. Computed CRFs for crashes and crash rates

Category of Improvement	CRF for all Crashes	CRF for I/F Crashes
Add two-way left turn lane	0.827	0.1994
Install new median barrier	0.8634	0.8837
Remove/relocate fixed object	0.3824	0.3814
Flatten slope, remove guard rail	0.5358	*
Flatten vertical curve	0.1956	0.5118

Provide interchange lighting	0.367	0.2696
Close median opening	6.17**	-3.19**

*Analysis could not be performed (see Section 4.3).

**Empirical Bayes method was not employed (see Section 4.7)

Better precision for the estimates of CRF could be achieved by adding more treatment sites to the study, along with similar comparison sites. This study was considerably smaller than the example studies in the references for the Empirical Bayes procedure, which typically include 10's or 100's of treatment sites and 1000's of comparison sites.

The following is an example of how CRFs could be used in safety studies. Say, in a highway section, average number of crashes per year is 90 and average ADT is 8200. It was determined that one of the causal factors for these crashes is sight distance obstruction by fixed objects, such as trees, utility poles, etc. It was decided to remove or relocate these fixed objects, so that sight distance will no longer be a problem for this section of the highway. ADT after the improvement is estimated to be 9500. By using the CRFs of 38.24% for all crashes, estimated reduction in total crashes could be computed using the following formula:

$$\begin{aligned}
 \text{Crashes Prevented} &= N \times CR \times [(\text{ADT after improvement})/(\text{ADT before improvement})] \\
 &= 90 \times 0.3824 \times (9500/8400) \\
 &= \text{about 39 crashes}
 \end{aligned}$$

6. Conclusions and Recommendations

The following seven improvement categories were selected based on ranking of importance to ODOT: add two-way left turn lanes, install median barrier, flatten slope/remove guardrail, remove/relocate fixed object, flatten vertical curve, provide interchange lighting and close median openings. An Empirical Bayes methodology was developed and applied in developing the CRFs for all crashes and injury/fatal crashes. By using the methodology, the regression-to-the-mean biases associated with simple before and after crash analysis was avoided.

The research team also compiled CRFs for other improvement categories from credible studies, although only a limited number of credible studies were available that can be adopted for Ohio. CRFs from many studies could not be selected because of the sample size, inconclusive results and methodology that did not consider the regression to the mean biases. Consequently, it was not possible to identify CRFs for all improvement categories that are of interest to ODOT.

The research team developed a statistical analysis tool using SAS for developing CRFs that can be easily applied to any improvement category that is of interest to ODOT. The research team recommends that ODOT develop their own reduction factors using the methodology developed through this research for other categories of improvements that are of importance to ODOT. This will provide the highest confidence in the application of the CRFs for future safety improvement projects and resource allocations.

7. References

Al-Masaeid, H. R., and K. C. Sinha, Analysis of Accident Reduction Potentials of Pavement Markings. *Journal of Transportation Engineering*, Vol. 120, No. 5, pp. 723-736 (Sept./Oct. 1994)

Bhesania, R.P. "Impact of Mast-Mounted Signal Heads on accident Reduction." *ITE Journal*, p. 25-29, October 1991.

Brian Bowman and Robert L. Vecello, "Effects of Urban and Suburban Median Types on Both Vehicular and Pedestrian Safety," *Transportation Research record*, 1445, 1994.

Ermer, O. J., *Accident Reduction Factors for Indiana*, Report No. FHWA/IN/JHRP-91/11, Joint Highway Research Program, Purdue University School of Engineering, Indiana Department of Transportation, May 1991.

Hanley, K.E., A. R. Gibby, and T. C. Ferrara, *The Evaluation of Accident Reduction Factors on California State Highways*, Report No. FHWA/CA-TO-96- , California Department of Transportation, Federal Highway Administration, December 1996.

Hauer, E., D.W. Harwood, F.M. Council, M.S. Griffith, The Empirical Bayes method for estimating safety: A tutorial. *Transportation Research Record 1784*, pp. 126-131. National Academies Press, Washington, D.C. 2002

Harwood, D., F. Council, E. Hauer, W. Hughes and A. Vogt, Prediction of the Expected Safety Performance of Rural Two-Lane Highways, Final Report, Report No. FHWA-RD-99-207, December 2000.

Lyon, C., A. Huq, B. Persaud and Steven Kodama, Development of safety Performance Functions for Signalized Intersections in a Large Urban Area and Application to Evaluation of Left Turn Priority Treatment, *Transportation Research Board 2005 Annual Meeting CD-ROM*.

Persaud B.N., Retting R.A., Garder P.E., and D. Lord, "Safety effect of roundabout conversions in the United States: Empirical Bayes observational before-after study," *Transport. Res. Rec.*, no. 1751, pp. 1-8, 2001.

Sayed, T., P. de Leur and J. Pump, Safety Impacts of Increased Traffic Signal backboards Conspicuity, *Transportation Research Board 2005 Annual Meeting CD-ROM*.

SAS 8.2, SAS Institute Inc., Cary, North Carolina

Thomas G.B., Smith D.J., Effectiveness of Roadway Safety Improvement, CTRE management project pp. 00-61, Iowa State University, Ames, Iowa.

William C. Taylor, Inkyu Lim, and Dale R. Lightizer, Effect on Crashes After Construction of Directional Median Crossovers, Transportation Research Record 1758, pp 30-35, National Academy Press, Washington D.C., 2001.

APPENDIX A - C++ Program for Data Extraction

C++ Program

```
#include<stdio.h>
#include<stdlib.h>
#include<iostream.h>
#include<fstream.h>
#include<ctype.h>
#include<string.h>
void main (void) {
char crash_type[50];
char severity_type[50];
int month;
int year;
int crash_year;
FILE* infile; \\ input file pointer
FILE* outfile; \\ output file pointer
int property_damage[12][7];
int injury_fatal[12][7];
for(int i=0;i<12;i++)
    for (int j=0 ;j<7 ;j++)
    {
        property_damage[i][j]=0;
        injury_fatal[i][j]=0;
    }
infile=fopen("data.txt", "r");
if(infile=NULL)
printf("File could not be opened \n");
outfile=fopen("output.txt","w");
if( outfile==NULL)
printf("File could not be opened \n");
cout<<"Enter the year"<<endl;
cin>>crash_year;
cout<<"The year entered is" <<crash_year<<endl;
while( (fscanf(infile,"%[^\\t] %[^\\t]%d%d",
crash_type,severity_type,&month,&year))!=EOF){
int i=0;
int j=0;
if(year==crash_year )
{
    while(isspace(crash_type[i]))
        i++;
    while(isspace(severity_type[j]))
        j++;
    if(crash_type[i]=='A ')
    {
        if( crash_type[i+ 2]=='T')
```

```

        {
            if(severity_type[j]=='P')
                property_damage[month-1][6]++;
else
injury_fatal[month-1][ 6]++;
        }
else
        {
if( severity_type[j]==' P')
property_damage[month-1][0]++;
else
injury_fatal[month -1][0]++;
        }
else if(crash_type[i]=='R')
    {
if(severity_type[j]==' P')
property_damage[month-1][1]++;
else
injury_fatal[ month-1][1]++;
    }
else if(crash_type[i]=='F')
    {
if(crash_type[i+1]==' A')
property_damage[month-1][6]++;
else
    {
if (severity_type [j]=='P')
property_damage[month-1][2]++;
else
injury_fatal[month-1][2]++;
    }
    }
else if(crash_type[i]=='S')
    {
if( severity_type[j]=='P')
property_damage[month-1][3]++;
else
injury_fatal[month-1][3]++;
    }
else if(crash_type[i]='H')
    {
if(severity_type[j]=='P')
property_damage[month-1][ 4 ]++;
else
injury_fatal[month-1][4]++;
    }

```

```

    }
    else if(crash_type[i]=='L')
    {
    if(severity_type [j]=='P')
    property _damage[month-1][5]++;
    else
    injury _fatal[ month-1][5]++;
    }
    else
    property_damage[month-1] [6]++;
} // if(crash_year=year)
} // while
fprintf(outfile,"Table of crash of the year %d is \n", crash_year);
fprintf(outfile,'-----\n');
fprintf(outfile,"A          R      F      S      H      L      o\n");
fprintf(outfile," I/PDO          I/PDO I/PDO I/PDO I/PDO I/PDO I/PDO \n\n");
for(i=0;i<12;i++ )
{
    for (int j=0;j<7;j++)
        fprintf(outfile,"%d/%d\t", injury_fatal [i ][j],property_damage[i] [j]);
    fprintf(outfile,"\n\n");
}
fclose(infile );
fclose( outfile);
}

```

Analysis Results for Install Two-Way Left Turn

APPENDIX B - EB Modeling Steps: Add Two-Way Left Turn Lanes

Analysis Results for Install Two-Way Left Turn

B

The SAS System 22:18 Tuesday, June 29, 2004 1

The GENMOD Procedure

Model Information

Data Set	SASUSER.TWLT516	
Distribution	Negative Binomial	
Link Function	Log	
Dependent Variable	Ttl_f_Pred	Ttl_f_Pred
Offset Variable	DY	DY
Observations Used	179	
Missing Values	12	

Class Level Information

Class	Levels	Values
TYP	33	C1-1 C1-10 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C1-9 C2-1 C2-10 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C2-9 C3-1 C3-10 C3-2 C3-3 C3-4 C3-5 C3-6 C3-7 C3-8 C3-9 T1-1 T2-1 T3-1
Treatment	2	C T
Midpoint	21	01MAR00:12:00:00 01MAR96:12:00:00 02MAR01:00:00:00 02MAR95:00:00:00 02MAR97:00:00:00 02MAR98:00:00:00 02MAR99:00:00:00 15JAN03:00:00:00 15JAN99:00:00:00 16APR98:12:00:00 16JUL02:12:00:00 16JUL98:12:00:00 16OCT97:00:00:00 30NOV00:00:00:00 ...
Construction	3	A N Y
CNTY	11	ADA(9) CLE(8) CRA(2) CUY(12) LOR(3) MUS(5) POR(4) VAN(1) WAS(10) WAY(1) clark (7)
MCL	3	1 2 4
SYS_CL	2	A M
ACS	2	L N
FC	5	6 7 14 16 17
AC	10	0 10 17 38 40 116 167 710 745 777
Lns	2	2 4

Parameter Information

Parameter	Effect
Prm1	Intercept
Prm2	lnADT

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	177	198.3515	1.1206
Scaled Deviance	177	198.3515	1.1206
Pearson Chi-Square	177	152.1625	0.8597

2

The SAS System 22:18 Tuesday, June 29, 2004

The GENMOD Procedure

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Scaled Pearson X2	177	152.1625	0.8597
Log Likelihood		747.6529	

Algorithm converged.

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald	95% Confidence Limits	Chi-Square	Pr > ChiSq
Intercept	1	-7.3310	1.3827	-10.0411	-4.6209	28.11	<.0001
lnADT	1	0.9613	0.1519	0.6636	1.2590	40.05	<.0001
Dispersion	1	1.0496	0.1495	0.7939	1.3876		

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

Analysis Results for Install Two-Way Left Turn

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (33 levels)
Number of Clusters	33
Clusters With Missing Values	3
Correlation Matrix Dimension	8
Maximum Cluster Size	7
Minimum Cluster Size	3

Algorithm converged.

Analysis Of GEE Parameter Estimates Empirical Standard Error Estimates

Parameter	Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
Intercept	-7.3310	1.9695	-11.1911	-3.4708	-3.72	0.0002
lnADT	0.9613	0.2226	0.5251	1.3975	4.32	<.0001

The SAS System 22:18 Tuesday, June 29, 2004

3

The GENMOD Procedure

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
lnADT	1	18.66	<.0001

Analysis Results for Install Two-Way Left Turn

Empirical Bayes (EB) Calculations for Total Crashes

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)	
T3-1	365	N	9750	4.476	1.000	7	25.506	0.175	6.557	5.406	6.818	1.948	
T3-1	366	N	9273	4.277	0.956	8	23.479	0.182	7.322	5.988			
T3-1	365	N	8797	4.055	0.906	6	21.311	0.190	5.630	4.559			
T3-1	365	Y	8320	3.843	0.859	5	19.346	0.199	4.770	3.823			
T3-1	92	Y	7843	0.915	0.204	2	1.794	0.510	1.447	0.709			
T3-1	273	A	7367	2.557	0.571	6	9.421	0.271	5.066	3.691	3.895	0.636	
T2-1	366	N	9372	4.321	1.000	11	23.920	0.181	9.793	8.024	6.352	1.765	
T2-1	365	N	9205	4.235	0.980	4	23.064	0.184	4.043	3.301			
T2-1	365	N	9011	4.150	0.960	5	22.222	0.187	4.841	3.937			
T2-1	92	Y	8816	1.024	0.237	2	2.125	0.482	1.530	0.792			
T2-1	273	A	8816	3.039	0.703	5	12.733	0.239	4.532	3.450	4.467	0.873	
T2-1	366	A	8599	3.978	0.921	7	20.588	0.193	6.416	5.176	5.848	1.496	
T2-1	365	A	8430	3.892	0.901	6	19.791	0.197	5.585	4.487	5.721	1.432	
T1-1	365	N	10015	4.593	1.000	8	26.736	0.172	7.415	6.141	6.968	1.953	
T1-1	366	N	9801	4.511	0.982	8	25.870	0.174	7.392	6.103			
T1-1	365	N	9640	4.428	0.964	6	25.004	0.177	5.722	4.708			
T1-1	91	Y	7162	0.830	0.181	2	1.552	0.535	1.374	0.640			
T1-1	274	A	10577	3.634	0.791	4	17.494	0.208	3.924	3.109	5.513	1.223	
T1-1	365	A	9264	4.261	0.928	4	23.323	0.183	4.048	3.308	6.465	1.681	
T1-1	366	A	9051	4.179	0.910	5	22.507	0.186	4.848	3.948	6.339	1.617	
T1-1	365	A	8888	4.095	0.892	4	21.697	0.189	4.018	3.260	6.212	1.553	
								SE					
	2647				Actual	41	41.000	6.403	38.436		44.460	10.510	3.242
									5.300	Theta	0.922		
	3288				Dispersion	1.0496			58.715	bias	1.005	Variance	Std Error
									6.518	Unbiased	0.917	0.025	0.157
									0.813				
Summary of CRF Calculations for Total Crashes										CRF	0.083		0.157
										Z	0.526		
										P-value	0.299		

Analysis Results for Install Two-Way Left Turn

The SAS System 09:08 Thursday, October 6, 2005 4

The GENMOD Procedure

Model Information

```

Data Set          SASUSER.TWLT516
Distribution      Negative Binomial
Link Function     Log
Dependent Variable IF_f_Pred
Offset Variable   DY
Observations Used 179
Missing Values    12
    
```

Class Level Information

Class	Levels	Values
TYP	33	C1-1 C1-10 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C1-9 C2-1 C2-10 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C2-9 C3-1 C3-10 C3-2 C3-3 C3-4 C3-5 C3-6 C3-7 C3-8 C3-9 T1-1 T2-1 T3-1
Treatment	2	C T
Midpoint	21	01MAR00:12:00:00 01MAR96:12:00:00 02MAR01:00:00:00 02MAR95:00:00:00 02MAR97:00:00:00 02MAR98:00:00:00 02MAR99:00:00:00 15JAN03:00:00:00 15JAN99:00:00:00 16APR98:12:00:00 16JUL02:12:00:00 16JUL98:12:00:00 16OCT97:00:00:00 30NOV00:00:00:00 ...
Construction	3	A N Y
CNTY	11	ADA(9) CLE(8) CRA(2) CUY(12) LOR(3) MUS(5) POR(4) VAN(1) WAS(10) WAY(1) clark (7)
MCL	3	1 2 4
SYS_CL	2	A M
ACS	2	L N
FC	5	6 7 14 16 17
AC	10	0 10 17 38 40 116 167 710 745 777
Lns	2	2 4

Parameter Information

Parameter	Effect
Prm1	Intercept
Prm2	lnADT

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	177	172.6681	0.9755
Scaled Deviance	177	172.6681	0.9755
Pearson Chi-Square	177	156.9418	0.8867

The SAS System 09:08 Thursday, October 6, 2005 5

The GENMOD Procedure

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Scaled Pearson X2	177	156.9418	0.8867
Log Likelihood		-100.6226	

Algorithm converged.

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald	95% Confidence Limits	Chi-Square	Pr > ChiSq
Intercept	1	-8.3417	1.7165	-11.7060	-4.9773	23.62	<.0001
lnADT	1	0.9335	0.1872	0.5667	1.3003	24.88	<.0001
Dispersion	1	1.1619	0.2651	0.7429	1.8172		

Analysis Results for Install Two-Way Left Turn

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (33 levels)
Number of Clusters	33
Clusters With Missing Values	3
Correlation Matrix Dimension	8
Maximum Cluster Size	7
Minimum Cluster Size	3

Algorithm converged.

Analysis Of GEE Parameter Estimates Empirical Standard Error Estimates

Parameter	Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
Intercept	-8.3417	2.3564	-12.9600	-3.7233	-3.54	0.0004
lnADT	0.9335	0.2623	0.4195	1.4476	3.56	0.0004

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
lnADT	1	12.67	0.0004

Analysis Results for Install Two-Way Left Turn

Empirical Bayes (EB) Calculations for Injury and Fatality Crashes

TYP	Duration	Construction	ADT	pIF	Cy	I_F	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)	
T3-1	365	N	9750	1.262	1.000	2	3.112	0.405	1.701	1.011	1.751	0.357	
T3-1	366	N	9273	1.207	0.957	3	2.901	0.416	2.254	1.316			
T3-1	365	N	8797	1.146	0.908	1	2.673	0.429	1.063	0.607			
T3-1	365	Y	8320	1.088	0.862	1	2.464	0.442	1.039	0.580			
T3-1	92	Y	7843	0.260	0.206	2	0.338	0.768	0.663	0.154			
T3-1	273	A	7367	0.727	0.576	2	1.340	0.542	1.309	0.599	1.008	0.118	
T2-1	366	N	9372	1.220	1.000	3	2.948	0.414	2.263	1.327	1.698	0.336	
T2-1	365	N	9205	1.196	0.981	1	2.858	0.418	1.082	0.629			
T2-1	365	N	9011	1.172	0.961	2	2.769	0.423	1.650	0.951			
T2-1	92	Y	8816	0.290	0.237	1	0.387	0.748	0.468	0.118			
T2-1	273	A	8816	0.859	0.704	2	1.717	0.500	1.429	0.714	1.196	0.167	
T2-1	366	A	8599	1.125	0.923	0	2.597	0.433	0.488	0.276	1.567	0.286	
T2-1	365	A	8430	1.102	0.903	3	2.512	0.439	2.167	1.217	1.534	0.274	
T1-1	365	N	10015	1.294	1.000	2	3.239	0.399	1.718	1.032	2.343	0.474	
T1-1	366	N	9801	1.272	0.983	5	3.150	0.404	3.495	2.084			
T1-1	365	N	9640	1.249	0.965	2	3.060	0.408	1.693	1.002			
T1-1	91	Y	7162	0.236	0.182	0	0.301	0.785	0.185	0.040			
T1-1	274	A	10577	1.022	0.790	1	2.236	0.457	1.010	0.548	1.851	0.296	
T1-1	365	A	9264	1.203	0.930	1	2.885	0.417	1.085	0.632	2.178	0.410	
T1-1	366	A	9051	1.180	0.912	2	2.800	0.422	1.654	0.957	2.138	0.395	
T1-1	365	A	8888	1.157	0.895	0	2.714	0.426	0.494	0.283	2.096	0.379	
	2647				Actual	11	11.000	SE	3.317	9.636	13.568	2.325	1.525
	3288								1.329	ThetaHat	0.811		
									16.919	bias	1.013	Variance	Std Error
									1.878	ThetaHatU	0.801	0.065	0.254
									0.708				
Summary of CRF Calculations for Total Crashes										CRF	0.199	0.254	
									Z	0.784			
									P-value	0.217			

APPENDIX C - EB Modeling Steps: Install Median Barriers

Analysis Results for Install Median Barrier

The SAS System

14:29 Tuesday, July 6, 2004 57

The GENMOD Procedure

Model Information

Data Set	WORK.DATA27		
Distribution	Negative Binomial		
Link Function	Log		
Dependent Variable	Total	Total	
Offset Variable	DY	DY	
Observations Used	292		

Class Level Information

Class	Levels	Values
TYP	44	C1-1 C1-10 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C1-9 C2-1 C2-10 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C2-9 C3-1 C3-10 C3-2 C3-3 C3-4 C3-5 C3-6 C3-7 C3-8 C3-9 C4-1 C4-10 C4-2 C4-3 C4-4 C4-5 C4-6 C4-7 C4-8 C4-9 T1-1 T2-1 T3-1 T4-1
Treatment	2	C T
Construction	1	N
CNTY	5	CUY(12) DEL(6) FRA(6) HAM(8) SUM(4)
MCL	1	2
SYS_CL	2	I M
ACS	2	F L
FC	2	11 12

Parameter Information

Parameter	Effect
Prm1	Intercept
Prm2	lnADT

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	290	341.3373	1.1770
Scaled Deviance	290	341.3373	1.1770
Pearson Chi-Square	290	324.1794	1.1179
Scaled Pearson X2	290	324.1794	1.1179
Log Likelihood		13520.7357	

Algorithm converged.

Analysis Results for Install Median Barrier

The SAS System 14:29 Tuesday, July 6, 2004 58

The GENMOD Procedure

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald 95% Confidence Limits		Chi-Square	Pr > ChiSq
Intercept	1	-19.6388	3.9017	-27.2860	-11.9915	25.33	<.0001
lnADT	1	1.9505	0.3388	1.2865	2.6145	33.15	<.0001
Dispersion	1	1.6183	0.1340	1.3760	1.9034		

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (44 levels)
Number of Clusters	44
Correlation Matrix Dimension	8
Maximum Cluster Size	8
Minimum Cluster Size	3

Algorithm converged.

Analysis Of GEE Parameter Estimates Empirical Standard Error Estimates

Parameter	Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
Intercept	-19.6388	5.9541	-31.3086	-7.9691	-3.30	0.0010
lnADT	1.9505	0.5129	0.9453	2.9557	3.80	0.0001

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
lnADT	1	14.46	0.0001

Analysis Results for Install Median Barrier

Empirical Bayes (EB) Calculations for Total Crashes

TYP	Duratio Constructio		ADT	ptotal	Cy	Total	V(count)	Alpha	EB		Projected	
	n	n							lambda	V(EB)	Count	V(PC)
T1-1	365	N	117000	25.44	1.00	0	1072.92	0.02	0.60	0.5890	3.44	1.0872
T1-1	366	N	118682	26.15	1.03	4	1132.59	0.02	4.51	4.4072		
T1-1	365	N	121014	26.97	1.06	5	1203.87	0.02	5.49	5.3691		
T1-1	365	Y	123021	27.74	1.09	8	1273.44	0.02	8.43	8.2465		
T1-1	365	Y	125028	28.53	1.12	5	1345.83	0.02	5.50	5.3823		
T1-1	213	Y	127034	17.11	0.67	3	491.06	0.03	3.49	3.3702		
T1-1	153	A	129041	12.63	0.50	1	270.77	0.05	1.54	1.4705	1.71	0.2679
T1-1	365	A	131050	30.95	1.22	10	1580.62	0.02	10.41	10.2063	4.18	1.6084
T1-1	365	A	133057	31.77	1.25	9	1664.96	0.02	9.43	9.2544	4.29	1.6951
T2-1	366	N	73932	11.55	1.00	52	227.34	0.05	49.95	47.4084	48.23	13.9434
T2-1	365	N	78191	12.69	1.10	69	273.10	0.05	66.38	63.3007		
T2-1	365	N	82247	13.84	1.20	44	323.96	0.04	42.71	40.8862		
T2-1	365	Y	86302	15.04	1.30	3	381.23	0.04	3.48	3.3381		
T2-1	366	Y	90111	16.25	1.41	9	443.68	0.04	9.27	8.9262		
T2-1	365	Y	94414	17.57	1.52	1	516.96	0.03	1.56	1.5098		
T2-1	153	Y	98469	7.92	0.69	0	109.38	0.07	0.57	0.5317		
T2-1	212	A	102524	11.76	1.02	0	235.70	0.05	0.59	0.5578	49.14	14.4695
T2-1	365	A	106581	21.66	1.88	0	780.68	0.03	0.60	0.5841	90.46	49.0432
T3-1	365	N	91057	8.58	1.00	2	127.75	0.07	2.44	2.2780	1.39	0.3980
T3-1	365	N	95630	9.34	1.09	1	150.48	0.06	1.52	1.4233		
T3-1	365	N	100202	10.12	1.18	0	175.96	0.06	0.58	0.5489		
T3-1	366	Y	104489	10.91	1.27	5	203.62	0.05	5.32	5.0319		
T3-1	365	Y	109347	11.77	1.37	5	236.00	0.05	5.34	5.0715		
T3-1	120	Y	113920	4.15	0.48	2	32.07	0.13	2.28	1.9838		
T3-1	245	A	104155	7.26	0.85	3	92.67	0.08	3.33	3.0729	1.18	0.2852

Analysis Results for Install Median Barrier

Empirical Bayes (EB) Calculations for Total Crashes (continued)

TYP	Duratio n	Constructio n	ADT	ptotal	Cy	Total V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)	
T4-1	365	N	82470	11.13	1.00	28	0.05	27.11	25.6856	21.32	6.5059	
T4-1	365	N	84180	11.53	1.04	27	0.05	26.21	24.8792			
T4-1	365	N	85890	11.94	1.07	13	0.05	12.95	12.3102			
T4-1	366	Y	87361	12.32	1.11	11	0.05	11.06	10.5350			
T4-1	365	Y	89310	12.77	1.15	4	0.05	4.40	4.2014			
T4-1	120	Y	91019	4.34	0.39	0	0.12	0.54	0.4734			
T4-1	245	A	91019	8.86	0.80	0	0.07	0.58	0.5399	16.97	4.1209	
	1950				ACTUAL	23	4.80	26.49		167.91	71.4901	
	4382				Dispersion	1.6183		4.96	theta	0.14		
								240.46	bias	1.00	Variance	
								20.03	Unbiased	0.14	0.0009	
								0.25			0.0292	
Summary of CRF Calculation for Total Crashes										CRF	0.86	0.0292
										Z	29.533	
										P-value	0.000	

Analysis Results for Install Median Barrier

The SAS System

15:40 Tuesday, July 6, 2004 53

The GENMOD Procedure

Model Information

Data Set	WORK.DATA26		
Distribution	Negative Binomial		
Link Function	Log		
Dependent Variable	Total	Total	
Offset Variable	DY	DY	
Observations Used	292		

Class Level Information

Class	Levels	Values
TYP	44	C1-1 C1-10 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C1-9 C2-1 C2-10 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C2-9 C3-1 C3-10 C3-2 C3-3 C3-4 C3-5 C3-6 C3-7 C3-8 C3-9 C4-1 C4-10 C4-2 C4-3 C4-4 C4-5 C4-6 C4-7 C4-8 C4-9 T1-1 T2-1 T3-1 T4-1
Treatment	2	C T
Construction	1	N
CNTY	5	CUY(12) DEL(6) FRA(6) HAM(8) SUM(4)
MCL	1	2
SYS_CL	2	I M
ACS	2	F L
FC	2	11 12

Parameter Information

Parameter	Effect
Prm1	Intercept
Prm2	lnADT
Prm3	Midpoint

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	289	323.5547	1.1196
Scaled Deviance	289	323.5547	1.1196
Pearson Chi-Square	289	333.1521	1.1528
Scaled Pearson X2	289	333.1521	1.1528
Log Likelihood		2291.9342	

Algorithm converged.

Analysis Results for Install Median Barrier

The SAS System 15:40 Tuesday, July 6, 2004 54

The GENMOD Procedure

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald 95% Confidence Limits		Chi-Square	Pr > ChiSq
Intercept	1	-20.7886	3.9369	-28.5048	-13.0724	27.88	<.0001
lnADT	1	2.0602	0.3449	1.3842	2.7362	35.68	<.0001
Midpoint	1	-0.0005	0.0001	-0.0007	-0.0003	19.80	<.0001
Dispersion	1	1.5766	0.1596	1.2929	1.9226		

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (44 levels)
Number of Clusters	44
Correlation Matrix Dimension	8
Maximum Cluster Size	8
Minimum Cluster Size	3

Algorithm converged.

Analysis Of GEE Parameter Estimates Empirical Standard Error Estimates

Parameter	Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
Intercept	-20.7886	6.9184	-34.3483	-7.2289	-3.00	0.0027
lnADT	2.0602	0.6055	0.8734	3.2470	3.40	0.0007
Midpoint	-0.0005	0.0001	-0.0007	-0.0002	-3.77	0.0002

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
lnADT	1	11.58	0.0007
Midpoint	1	14.23	0.0002

Analysis Results for Install Median Barrier

Empirical Bayes (EB) Calculations for Injury and Fatality Crashes

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB		Projected	
									lambda	V(EB)	Count	V(PC)
T1-1	365	N	117000	13.79	1.00	0	313.582	0.04	0.61	0.580	1.70	0.592
T1-1	366	N	118682	12.51	0.91	1	259.231	0.05	1.56	1.480		
T1-1	365	N	121014	11.39	0.83	2	215.948	0.05	2.50	2.364		
T1-1	365	Y	123021	10.35	0.75	5	179.126	0.06	5.31	5.002		
T1-1	365	Y	125028	9.39	0.68	1	148.522	0.06	1.53	1.434		
T1-1	213	Y	127034	5.11	0.37	0	46.214	0.11	0.56	0.502		
T1-1	153	A	129041	3.54	0.26	0	23.316	0.15	0.54	0.456	0.44	0.039
T1-1	365	A	131050	7.94	0.58	3	107.454	0.07	3.37	3.117	0.98	0.197
T1-1	365	A	133057	7.20	0.52	0	88.943	0.08	0.58	0.536	0.89	0.161
T2-1	366	N	73932	5.53	1.00	27	53.708	0.10	24.79	22.238	27.64	8.497
T2-1	365	N	78191	5.37	0.97	36	50.832	0.11	32.76	29.303		
T2-1	365	N	82247	5.18	0.94	25	47.507	0.11	22.84	20.348		
T2-1	365	Y	86302	4.98	0.90	1	44.044	0.11	1.45	1.286		
T2-1	366	Y	90111	4.75	0.86	5	40.368	0.12	4.97	4.386		
T2-1	365	Y	94414	4.54	0.82	0	37.052	0.12	0.56	0.488		
T2-1	153	Y	98469	1.88	0.34	0	7.426	0.25	0.47	0.354		
T2-1	212	A	102524	2.62	0.47	0	13.467	0.19	0.51	0.411	13.11	1.913
T2-1	365	A	106581	4.38	0.79	0	34.636	0.13	0.55	0.484	21.90	5.336
T3-1	365	N	91057	3.43	1.00	1	22.009	0.16	1.38	1.164	1.14	0.331
T3-1	365	N	95630	3.30	0.96	1	20.502	0.16	1.37	1.150		
T3-1	365	N	100202	3.17	0.92	0	18.960	0.17	0.53	0.440		
T3-1	366	Y	104489	3.02	0.88	4	17.349	0.17	3.83	3.163		
T3-1	365	Y	109347	2.87	0.84	2	15.904	0.18	2.16	1.768		
T3-1	120	Y	113920	0.94	0.27	1	2.313	0.40	0.97	0.580		
T3-1	245	A	104155	1.53	0.45	2	5.222	0.29	1.86	1.317	0.51	0.066

Analysis Results for Install Median Barrier

Empirical Bayes (EB) Calculations for Injury and Fatality Crashes (Continued)

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)	
T4-1	365	N	82470	4.43	1.00	14	35.398	0.13	12.80	11.199	11.92	3.752	
T4-1	365	N	84180	4.05	0.91	12	29.976	0.14	10.93	9.447			
T4-1	365	N	85890	3.71	0.84	10	25.374	0.15	9.08	7.754			
T4-1	366	Y	87361	3.38	0.76	2	21.382	0.16	2.22	1.867			
T4-1	365	Y	89310	3.09	0.70	1	18.155	0.17	1.36	1.125			
T4-1	120	Y	91019	0.97	0.22	0	2.441	0.40	0.38	0.231			
T4-1	245	A	91019	1.85	0.42	0	7.277	0.25	0.47	0.352	4.99	0.657	
								SE					
	1950					5	5	2.236	7.89		42.82	8.106	2.85
									1.48	theta	0.12		
	4382								121.14	bias	1.0044	Variance	Std Error
									10.09	Jnbiasec	0.12	0.003	0.05
									0.15				
Summary of CRF Calculation for Injury and Fatality Crashes													
										CRF	0.88		0.05
										Z	16.888		
										P-value	0.000		

APPENDIX D - EB Modeling Steps: Flatten Slope, Remove Guardrail

Analysis Results for Flatten Slope Remove Guard Rail

The SAS System

11:44 Tuesday, July 20, 2004 15

The GENMOD Procedure

Model Information

Data Set	WORK.DATA9		
Distribution	Negative Binomial		
Link Function	Log		
Dependent Variable	Total	Total	
Offset Variable	DY	DY	
Observations Used	86		

Class Level Information

Class	Levels	Values
TYP	22	C1-1 C1-10 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C1-9 C2-1 C2-10 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C2-9 T1-1 T2-1
Treatment	2	C T
Construction	1	N
CNTY	1	HAN
MCL	2	1 2
SYS_CL	3	A L M
ACS	1	N
FC	3	6 7 8
AC	1	0

Parameter Information

Parameter	Effect
Prm1	Intercept
Prm2	lnTrcks
Prm3	lnADT

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	83	70.4039	0.8482
Scaled Deviance	83	70.4039	0.8482
Pearson Chi-Square	83	93.6820	1.1287
Scaled Pearson X2	83	93.6820	1.1287
Log Likelihood		-53.1391	

Algorithm converged.

Analysis Results for Flatten Slope Remove Guard Rail

The SAS System

11:44 Tuesday, July 20, 2004 16

The GENMOD Procedure

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald 95% Confidence Limits		Chi-Square	Pr > ChiSq
Intercept	1	-6.4369	1.5611	-9.4966	-3.3773	17.00	<.0001
lnTrcks	1	0.6545	0.4034	-0.1361	1.4452	2.63	0.1047
lnADT	1	0.5703	0.2013	0.1757	0.9648	8.02	0.0046
Dispersion	1	0.0019	0.5190	0.0000	1.52E230		

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (22 levels)
Number of Clusters	22
Correlation Matrix Dimension	4
Maximum Cluster Size	4
Minimum Cluster Size	3

Algorithm converged.

Analysis Of GEE Parameter Estimates Empirical Standard Error Estimates

Parameter	Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
Intercept	-6.4369	1.6699	-9.7098	-3.1641	-3.85	0.0001
lnTrcks	0.6545	0.1881	0.2858	1.0233	3.48	0.0005
lnADT	0.5703	0.2158	0.1473	0.9932	2.64	0.0082

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
lnTrcks	1	12.11	0.0005
lnADT	1	6.98	0.0082

Analysis Results for Flatten Slope Remove Guard Rail

Empirical Bayes (EB) Calculations for Total Crashes

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)		
T1-1	366	N	4099	1.21	1.00	2	1.217	0.998	1.22	0.003	1.21	0.001		
T1-1	365	N	4260	1.25	1.03	0	1.249	0.998	1.24	0.003				
T1-1	365	N	4410	1.28	1.05	2	1.281	0.998	1.28	0.003				
T1-1	92	Y	4563	0.33	0.27	0	0.331	0.999	0.33	0.000				
T1-1	273	A	4559	0.98	0.81	0	0.982	0.998	0.98	0.002	0.98	0.001		
T2-1	365	N	2160	0.41	1.00	0	0.414	0.999	0.41	0.000	0.41	0.000		
T2-1	366	N	2154	0.41	1.00	0	0.414	0.999	0.41	0.000				
T2-1	365	N	2160	0.41	1.00	0	0.414	0.999	0.41	0.000				
T2-1	61	Y	2154	0.07	0.17	0	0.069	1.000	0.07	0.000				
T2-1	304	A	2161	0.34	0.83	0	0.345	0.999	0.34	0.000	0.34	0.000		
T2-1	365	A	2161	0.41	1.00	1	0.414	0.999	0.41	0.000	0.41	0.000		
SE														
	577				Actual	1		1	1	1.32	1.74	0.001	0.028	
										0.84	theta	0.58		
	2192				Dispersion	0.0019				4.98	bias	1.00	Variance	Std Error
										0.83	Unbiased	0.58	0.331	0.575
										1.01				
Summary of CRF Calculations for Total Crashes.										CRF	0.425	0.575		
											Z	0.738		
											P-value	0.230		

APPENDIX E - EB Modeling Steps: Remove/Relocate Fixed Objects

Analysis Results for Remove/Relocate Fixed object

The SAS System

11:09 Monday, June 28, 2004 1

The GENMOD Procedure

Model Information

Data Set	WORK.DATA1		
Distribution	Negative Binomial		
Link Function	Log		
Dependent Variable	Total	Total	
Offset Variable	DY	DY	
Observations Used	262		

Class Level Information

Class	Levels	Values
TYP	44	C1-1 C1-10 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C1-9 C2-1 C2-10 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C2-9 C3-1 C3-10 C3-2 C3-3 C3-4 C3-5 C3-6 C3-7 C3-8 C3-9 C4-1 C4-10 C4-2 C4-3 C4-4 C4-5 C4-6 C4-7 C4-8 C4-9 T1-1 T2-1 T3-1 T4-1
Treatment	2	C T
Construction	1	N
CNTY	10	ADA(9) BRO(9) BUT(8) CHP(7) CLA(7) CLE(8) HAM(8) MOT(7) PIK(9) WAR(8)
MCL	3	1 2 4
SYS_CL	3	A L M
ACS	2	L N
FC	5	6 7 8 14 16

Parameter Information

Parameter	Effect	ACS	FC
Prm1	Intercept		
Prm2	FC		6
Prm3	FC		7
Prm4	FC		8
Prm5	FC		14
Prm6	FC		16
Prm7	lnADT		
Prm8	lnSW		
Prm9	ACS	L	
Prm10	ACS	N	
Prm11	lnTrcks		

Analysis Results for Remove/Relocate Fixed object

The SAS System

11:09 Monday, June 28, 2004 2

The GENMOD Procedure

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	253	275.9814	1.0908
Scaled Deviance	253	275.9814	1.0908
Pearson Chi-Square	253	276.7846	1.0940
Scaled Pearson X2	253	276.7846	1.0940
Log Likelihood		462.2745	

Algorithm converged.

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald 95% Confidence Limits		Chi-Square	Pr > ChiSq
Intercept	1	1.4827	0.9875	-0.4527	3.4181	2.25	0.1332
FC	6	2.1956	0.4342	1.3445	3.0467	25.57	<.0001
FC	7	-0.2449	0.1685	-0.5751	0.0853	2.11	0.1461
FC	8	-1.3426	0.4192	-2.1642	-0.5210	10.26	0.0014
FC	14	0.5334	0.2471	0.0491	1.0177	4.66	0.0309
FC	16	0.0000	0.0000	0.0000	0.0000	.	.
lnADT	1	0.1995	0.1024	-0.0012	0.4001	3.80	0.0513
lnSW	1	-1.1324	0.1516	-1.4296	-0.8352	55.78	<.0001
ACS	L	-0.6847	0.3766	-1.4227	0.0534	3.31	0.0690
ACS	N	0.0000	0.0000	0.0000	0.0000	.	.
lnTrcks	1	-0.4754	0.1665	-0.8018	-0.1491	8.15	0.0043
Dispersion	1	0.2597	0.0573	0.1686	0.4001		

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (44 levels)
Number of Clusters	44
Correlation Matrix Dimension	8
Maximum Cluster Size	8
Minimum Cluster Size	3

Algorithm converged.

Analysis Results for Remove/Relocate Fixed object

The SAS System

11:09 Monday, June 28, 2004 3

The GENMOD Procedure

Analysis Of GEE Parameter Estimates Empirical Standard Error Estimates

Parameter		Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
Intercept		1.4827	1.5536	-1.5623	4.5277	0.95	0.3399
FC	6	2.1956	0.3384	1.5323	2.8589	6.49	<.0001
FC	7	-0.2449	0.3338	-0.8992	0.4093	-0.73	0.4631
FC	8	-1.3426	0.6818	-2.6790	-0.0062	-1.97	0.0489
FC	14	0.5334	0.4462	-0.3411	1.4079	1.20	0.2319
FC	16	0.0000	0.0000	0.0000	0.0000	.	.
lnADT		0.1995	0.1730	-0.1396	0.5386	1.15	0.2489
lnSW		-1.1324	0.3188	-1.7573	-0.5075	-3.55	0.0004
ACS	L	-0.6847	0.0701	-0.8222	-0.5472	-9.76	<.0001
ACS	N	0.0000	0.0000	0.0000	0.0000	.	.
lnTrcks		-0.4754	0.2213	-0.9091	-0.0418	-2.15	0.0317

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
FC	4	124.37	<.0001
lnADT	1	1.33	0.2489
lnSW	1	12.61	0.0004
ACS	1	95.27	<.0001
lnTrcks	1	4.62	0.0317

Analysis Results for Remove/Relocate Fixed object

Empirical Bayes (EB) Calculations for Total Crashes

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB		Projected	
									lambda	V(EB)	Count	V(PC)
T1-1	365	N	1212	1.550797	1.00	1	2.18	0.7129	1.393	0.400	1.393	0.133
T1-1	366	N	1252	1.552225	1.00	1	2.18	0.7127	1.394	0.400		
T1-1	365	N	1298	1.545872	1.00	1	2.17	0.7135	1.390	0.398		
T1-1	31	Y	1648	0.133769	0.09	0	0.14	0.9664	0.129	0.004		
T1-1	334	A	1341	1.413528	0.91	1	1.93	0.7315	1.302	0.350	1.270	0.111
T1-1	365	A	1384	1.542049	0.99	0	2.16	0.714	1.101	0.315	1.385	0.132
T1-1	366	A	1423	1.544548	1.00	1	2.16	0.7137	1.389	0.398	1.387	0.132
T2-1	365	N	9360	8.66564	1.00	24	28.17	0.3076	19.282	13.350	15.844	3.716
T2-1	365	N	9015	8.472526	0.98	17	27.11	0.3125	14.335	9.856		
T2-1	365	N	8670	8.2883	0.96	15	26.13	0.3172	12.871	8.788		
T2-1	122	Y	8323	2.70468	0.31	2	4.60	0.5874	2.414	0.996		
T2-1	244	A	8325	5.409451	0.62	8	13.01	0.4158	6.923	4.044	9.890	1.448
T2-1	365	A	7980	7.886553	0.91	7	24.04	0.3281	7.291	4.899	14.419	3.078
T2-1	365	A	7635	7.674466	0.89	8	22.97	0.3341	7.891	5.255	14.032	2.915
T3-1	365	N	4474	7.939175	1.00	6	24.31	0.3266	6.633	4.467	7.942	1.774
T3-1	365	N	4760	7.991215	1.01	10	24.58	0.3252	9.347	6.308		
T3-1	366	N	5032	8.060822	1.02	8	24.94	0.3233	8.020	5.427		
T3-1	365	Y	5333	8.082816	1.02	11	25.05	0.3227	10.059	6.813		
T3-1	365	Y	5620	8.128511	1.02	9	25.29	0.3214	8.720	5.917		
T3-1	123	Y	5905	2.753933	0.35	3	4.72	0.583	2.857	1.191		
T3-1	242	A	5905	5.41827	0.68	3	13.04	0.4154	4.005	2.341	5.420	0.826
T3-1	366	A	6176	8.235098	1.04	10	25.85	0.3186	9.438	6.431	8.238	1.909
T3-1	365	A	6480	8.24979	1.04	2	25.92	0.3182	3.989	2.719	8.253	1.916

Analysis Results for Remove/Relocate Fixed object

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)	
T4-1	365	N	1026	0.147932	1.00	0	0.15	0.963	0.142	0.005	0.142	0.002	
T4-1	365	N	1035	0.148824	1.01	0	0.15	0.9628	0.143	0.005			
T4-1	366	N	1040	0.149889	1.01	0	0.16	0.9625	0.144	0.005			
T4-1	31	Y	1048	0.012755	0.09	1	0.01	0.9967	0.016	0.000			
T4-1	334	A	1043	0.137357	0.93	0	0.14	0.9656	0.133	0.005	0.132	0.002	
T4-1	365	A	1061	0.150955	1.02	0	0.16	0.9623	0.145	0.005	0.145	0.002	
	3711				Actual	40	40	6.325	43.606		64.572	12.470	3.531
	4383								4.289	theta	0.619		
					Dispersion		0.2597		75.094	bias	1.003	Variance	Std Error
									6.254	Unbiased	0.618	0.011	0.103
									0.686				
Summary of CRF Calculation for Total Crashes										CRF	0.382	0.103	
										Z	3.712		
										P-value	0.000		

Analysis Results for Remove/Relocate Fixed object

The SAS System

11:14 Monday, June 28, 2004 1

The GENMOD Procedure

Model Information

Data Set	WORK.DATA1		
Distribution	Negative Binomial		
Link Function	Log		
Dependent Variable	Total	Total	
Offset Variable	DY	DY	
Observations Used	262		

Class Level Information

Class	Levels	Values
TYP	44	C1-1 C1-10 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C1-9 C2-1 C2-10 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C2-9 C3-1 C3-10 C3-2 C3-3 C3-4 C3-5 C3-6 C3-7 C3-8 C3-9 C4-1 C4-10 C4-2 C4-3 C4-4 C4-5 C4-6 C4-7 C4-8 C4-9 T1-1 T2-1 T3-1 T4-1
Treatment	2	C T
Construction	1	N
CNTY	10	ADA(9) BRO(9) BUT(8) CHP(7) CLA(7) CLE(8) HAM(8) MOT(7) PIK(9) WAR(8)
MCL	3	1 2 4
SYS_CL	3	A L M
ACS	2	L N
FC	5	6 7 8 14 16

Parameter Information

Parameter	Effect	ACS	FC
Prm1	Intercept		
Prm2	FC		6
Prm3	FC		7
Prm4	FC		8
Prm5	FC		14
Prm6	FC		16
Prm7	lnADT		
Prm8	lnSW		
Prm9	ACS	L	
Prm10	ACS	N	

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	254	274.4632	1.0806
Scaled Deviance	254	274.4632	1.0806

Analysis Results for Remove/Relocate Fixed object

The SAS System

11:14 Monday, June 28, 2004 2

The GENMOD Procedure

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Pearson Chi-Square	254	308.1935	1.2134
Scaled Pearson X2	254	308.1935	1.2134
Log Likelihood		31.1682	

Algorithm converged.

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald 95% Confidence Limits		Chi-Square	Pr > ChiSq
Intercept	1	-1.1020	0.9256	-2.9160	0.7121	1.42	0.2338
FC	6	2.2433	0.4485	1.3643	3.1223	25.02	<.0001
FC	7	-0.1002	0.1872	-0.4671	0.2667	0.29	0.5925
FC	8	-0.8018	0.4822	-1.7468	0.1432	2.77	0.0963
FC	14	0.4796	0.2607	-0.0313	0.9905	3.38	0.0658
FC	16	0.0000	0.0000	0.0000	0.0000	.	.
lnADT	1	0.4051	0.1003	0.2085	0.6017	16.31	<.0001
lnSW	1	-1.4265	0.1721	-1.7638	-1.0891	68.67	<.0001
ACS	L	-0.6406	0.3803	-1.3859	0.1047	2.84	0.0921
ACS	N	0.0000	0.0000	0.0000	0.0000	.	.
Dispersion	1	0.1908	0.0653	0.0975	0.3731		

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (44 levels)
Number of Clusters	44
Correlation Matrix Dimension	8
Maximum Cluster Size	8
Minimum Cluster Size	3

Algorithm converged.

Analysis Results for Remove/Relocate Fixed object

The SAS System

11:14 Monday, June 28, 2004 3

The GENMOD Procedure

Analysis Of GEE Parameter Estimates
Empirical Standard Error Estimates

Parameter		Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
Intercept		-1.1020	1.3856	-3.8176	1.6137	-0.80	0.4264
FC	6	2.2433	0.5217	1.2208	3.2658	4.30	<.0001
FC	7	-0.1002	0.4256	-0.9344	0.7340	-0.24	0.8139
FC	8	-0.8018	0.7185	-2.2100	0.6064	-1.12	0.2644
FC	14	0.4796	0.5198	-0.5392	1.4983	0.92	0.3562
FC	16	0.0000	0.0000	0.0000	0.0000	.	.
lnADT		0.4051	0.1689	0.0741	0.7361	2.40	0.0165
lnSW		-1.4265	0.3440	-2.1008	-0.7521	-4.15	<.0001
ACS	L	-0.6406	0.1316	-0.8986	-0.3825	-4.87	<.0001
ACS	N	0.0000	0.0000	0.0000	0.0000	.	.

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
FC	4	85.45	<.0001
lnADT	1	5.75	0.0165
lnSW	1	17.19	<.0001
ACS	1	23.68	<.0001

Analysis Results for Remove/Relocate Fixed object

Empirical Bayes (EB) Calculations for Injury and Fatality Crashes

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB		Projected	
									lambda	V(EB)	Count	V(PC)
T1-1	365	N	1212	0.811	1.000	0	0.937	0.866	0.702	0.094	0.701	0.031
T1-1	366	N	1252	0.821	1.012	0	0.950	0.865	0.710	0.096		
T1-1	365	N	1298	0.828	1.020	0	0.958	0.864	0.715	0.097		
T1-1	31	Y	1648	0.075	0.093	0	0.076	0.986	0.074	0.001		
T1-1	334	A	1341	0.765	0.943	1	0.876	0.873	0.795	0.101	0.661	0.028
T1-1	365	A	1384	0.843	1.040	0	0.979	0.861	0.727	0.101	0.729	0.034
T1-1	366	A	1423	0.853	1.051	0	0.991	0.860	0.733	0.103	0.737	0.035
T2-1	365	N	9360	4.670	1.000	17	8.832	0.529	10.480	4.938	8.016	1.267
T2-1	365	N	9015	4.619	0.989	9	8.690	0.532	6.671	3.125		
T2-1	365	N	8670	4.566	0.978	9	8.545	0.534	6.631	3.087		
T2-1	122	Y	8323	1.508	0.323	1	1.942	0.777	1.395	0.312		
T2-1	244	A	8325	3.016	0.646	6	4.752	0.635	4.106	1.500	5.177	0.528
T2-1	365	A	7980	4.457	0.954	5	8.246	0.540	4.706	2.163	7.649	1.154
T2-1	365	A	7635	4.399	0.942	5	8.091	0.544	4.673	2.133	7.551	1.124
T3-1	365	N	4474	4.893	1.000	2	9.462	0.517	3.496	1.688	4.908	0.784
T3-1	365	N	4760	4.983	1.018	6	9.721	0.513	5.479	2.670		
T3-1	366	N	5032	5.079	1.038	7	10.001	0.508	6.024	2.965		
T3-1	365	Y	5333	5.152	1.053	8	10.217	0.504	6.564	3.254		
T3-1	365	Y	5620	5.232	1.069	4	10.455	0.500	4.617	2.306		
T3-1	123	Y	5905	1.789	0.366	1	2.400	0.746	1.588	0.404		
T3-1	242	A	5905	3.520	0.719	1	5.883	0.598	2.507	1.007	3.530	0.406
T3-1	366	A	6176	5.394	1.102	4	10.945	0.493	4.687	2.377	5.410	0.953
T3-1	365	A	6480	5.456	1.115	1	11.134	0.490	3.183	1.623	5.472	0.975

Analysis Results for Remove/Relocate Fixed object

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)	
T4-1	365	N	1026	0.064	1.000	0	0.065	0.988	0.064	0.001	0.064	0.000	
T4-1	365	N	1035	0.065	1.003	0	0.065	0.988	0.064	0.001			
T4-1	366	N	1040	0.065	1.007	0	0.066	0.988	0.064	0.001			
T4-1	31	Y	1048	0.006	0.085	1	0.006	0.999	0.007	0.000			
T4-1	334	A	1043	0.059	0.919	0	0.060	0.989	0.059	0.001	0.059	0.000	
T4-1	365	A	1061	0.065	1.010	0	0.066	0.988	0.064	0.001	0.064	0.000	
	3711				Actual	23	23.000	4.796	26.241		37.039	5.235	2.288
	4383				Dispersion		0.1908		2.581	theta	0.621		
									41.100	bias	1.004	Variance	Std Error
									3.423	Unbiased	0.619	0.018	0.134
									0.754				
Summary of CRF Calculation for Injury and Fatality Crashes										CRF	38.140	13.402	
										Z	2.846		
										P-value	0.002		

APPENDIX F - EB Modeling Steps: Flatten Vertical Curve

Analysis Results for Flatten Vertical Curve

The SAS System

13:44 Friday, July 2, 2004 1

The GENMOD Procedure

Model Information

Data Set	SASUSER.FVC_DATA_MDPTSQ		
Distribution	Negative Binomial		
Link Function	Log		
Dependent Variable	T_pred	T_pred	
Offset Variable	DY	DY	
Observations Used	348		
Missing Values	20		

Class Level Information

Class	Levels	Values
TYP	52	C1-1 C1-10 C1-11 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C1-9 C2-1 C2-10 C2-11 C2-12 C2-13 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C2-9 C3-1 C3-10 C3-11 C3-12 C3-2 C3-3 C3-4 C3-5 C3-6 C3-7 C3-8 C3-9 C4-1 C4-10 C4-11 C4-12 C4-2 C4-3 C4-4 C4-5 C4-6 C4-7 C4-8 C4-9 ...
Treatment	2	C T
TG	4	1 2 3 4
Construction	3	A N Y
CNTY	6	DEF(1) FUL(2) LIC(5) MUS(5) RIC(3) WAS(10)
MCL	3	1 2 4
SYS_CL	3	A L M
ACS	1	N
FC	4	2 6 7 8
Lns	1	2

Parameter Information

Parameter	Effect	SYS_CL
Prm1	Intercept	
Prm2	lnADT	
Prm3	lnSL	
Prm4	SYS_CL	A
Prm5	SYS_CL	L
Prm6	SYS_CL	M

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	343	325.3723	0.9486
Scaled Deviance	343	325.3723	0.9486
Pearson Chi-Square	343	459.7782	1.3405

Analysis Results for Flatten Vertical Curve

The SAS System

13:44 Friday, July 2, 2004 2

The GENMOD Procedure

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Scaled Pearson X2	343	459.7782	1.3405
Log Likelihood		-111.8764	

Algorithm converged.

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald	95% Confidence Limits	Chi-Square	Pr > ChiSq
Intercept	1	-2.9451	0.9282	-4.7644	-1.1257	10.07	0.0015
lnADT	1	0.5470	0.0998	0.3515	0.7425	30.07	<.0001
lnSL	1	0.8849	0.1811	0.5299	1.2400	23.87	<.0001
SYS_CL	A 1	-0.8205	0.3246	-1.4567	-0.1842	6.39	0.0115
SYS_CL	L 1	-0.1827	0.3089	-0.7881	0.4227	0.35	0.5541
SYS_CL	M 0	0.0000	0.0000	0.0000	0.0000	.	.
Dispersion	1	1.3762	0.2078	1.0237	1.8501		

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (52 levels)
Number of Clusters	52
Clusters With Missing Values	4
Correlation Matrix Dimension	8
Maximum Cluster Size	7
Minimum Cluster Size	3

Algorithm converged.

Analysis Of GEE Parameter Estimates Empirical Standard Error Estimates

Parameter	Estimate	Standard Error	95% Confidence Limits	Z	Pr > Z
Intercept	-2.9451	1.3828	-5.6553 -0.2349	-2.13	0.0332
lnADT	0.5470	0.1534	0.2463 0.8477	3.57	0.0004
lnSL	0.8849	0.3265	0.2450 1.5249	2.71	0.0067
SYS_CL	A -0.8205	0.3607	-1.5275 -0.1134	-2.27	0.0229

Analysis Results for Flatten Vertical Curve

The SAS System

13:44 Friday, July 2, 2004 3

The GENMOD Procedure

Analysis Of GEE Parameter Estimates
Empirical Standard Error Estimates

Parameter		Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
SYS_CL	L	-0.1827	0.4209	-1.0076	0.6422	-0.43	0.6642
SYS_CL	M	0.0000	0.0000	0.0000	0.0000	.	.

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
lnADT	1	12.71	0.0004
lnSL	1	7.35	0.0067
SYS_CL	2	9.18	0.0102

Analysis Results for Flatten Vertical Curve

Empirical Bayes (EB) Calculations for Total Crashes

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB		Projected	
									lambda	V(EB)	Count	V(PC)
T1-1	365	N	1693	0.732	1.000	3	1.469	0.498	1.870	0.938	1.502	0.246
T1-1	366	N	1805	0.760	1.039	3	1.555	0.489	1.905	0.974		
T1-1	365	N	1918	0.784	1.071	1	1.628	0.481	0.896	0.465		
T1-1	212	Y	1918	0.515	0.704	0	0.881	0.585	0.301	0.125		
T1-1	153	A	2030	0.452	0.618	0	0.733	0.616	0.279	0.107	0.928	0.094
T1-1	365	A	2145	0.833	1.138	0	1.788	0.466	0.388	0.207	1.710	0.319
T1-1	366	A	2260	0.859	1.174	2	1.876	0.458	1.477	0.801	1.764	0.339
T1-1	365	A	2160	0.836	1.143	5	1.798	0.465	3.064	1.639	1.716	0.321
T2-1	365	N	3935	2.581	1.000	7	11.749	0.220	6.029	4.705	5.022	1.250
T2-1	366	N	4360	2.738	1.061	8	13.051	0.210	6.896	5.450		
T2-1	365	N	4720	2.851	1.105	3	14.038	0.203	2.970	2.367		
T2-1	61	Y	4720	1.240	0.480	0	3.355	0.370	0.458	0.289		
T2-1	304	A	5080	2.511	0.973	6	11.191	0.224	5.217	4.046	4.886	1.183
T2-1	365	A	5440	3.081	1.194	14	16.148	0.191	11.917	9.643	5.995	1.781
T2-1	366	A	5800	3.200	1.240	9	17.293	0.185	7.927	6.460	6.226	1.921
T2-1	365	A	6160	3.298	1.278	10	18.269	0.181	8.790	7.203	6.417	2.041
T3-1	366	N	1300	1.202	1.000	1	3.189	0.377	1.076	0.671	0.888	0.189
T3-1	365	N	1220	1.158	0.963	0	3.001	0.386	0.446	0.274		
T3-1	365	N	1140	1.115	0.928	1	2.827	0.394	1.046	0.633		
T3-1	31	Y	1060	0.429	0.357	0	0.683	0.629	0.270	0.100		
T3-1	334	A	980	0.943	0.785	1	2.167	0.435	0.975	0.551	0.697	0.116
T3-1	366	A	980	1.030	0.857	0	2.489	0.414	0.426	0.250	0.761	0.139
T3-1	365	A	990	1.033	0.859	0	2.500	0.413	0.426	0.250	0.763	0.139
T3-1	365	A	1000	1.038	0.864	0	2.522	0.412	0.427	0.251	0.767	0.141

Analysis Results for Flatten Vertical Curve

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)	
T4-1	366	N	7830	0.927	1.000	4	2.110	0.439	2.650	1.486	4.534	0.850	
T4-1	365	N	7710	0.917	0.989	10	2.074	0.442	5.984	3.339			
T4-1	365	N	7712	0.917	0.989	8	2.074	0.442	4.869	2.716			
T4-1	275	Y	7713	0.717	0.773	3	1.424	0.503	1.850	0.919			
T4-1	90	A	7715	0.432	0.466	4	0.688	0.627	1.762	0.657	2.112	0.184	
T4-1	366	A	7715	0.920	0.992	2	2.084	0.441	1.523	0.851	4.498	0.837	
T4-1	365	A	7717	0.917	0.989	4	2.076	0.442	2.638	1.472	4.486	0.832	
T4-1	365	A	7718	0.917	0.989	2	2.076	0.442	1.522	0.849	4.487	0.832	
	5265	A			Actual	59	59	7.681	48.758		48.214	11.219	3.350
	4384	N			Dispersic	1.3762			3.380	ThetaHat	1.224		
									36.637	bias	1.005	Variance	Std Error
									3.050	GammaHat	1.218	0.032	0.179
									1.108				

Summary of CRF Calculations for Total Crashes

All Treatment Sites	CRF	-0.218	0.179
	Z	-1.218	
	P-value	0.888	
	SE		
20	20	4.472	40.430
			24.689
			4.293
			3.380
			0.810
			113.564
			1.007
			Variance
			Std Error
			0.804
			0.036
			0.191
			1.108
All Treatment Sites Except for T2	CRF	0.196	0.191
	Z	1.025	
	P-value	0.153	

Analysis Results for Flatten Vertical Curve

The SAS System

14:55 Friday, July 2, 2004 1

The GENMOD Procedure

Model Information

Data Set	SASUSER.FVC_DATA_MDPTSQ		
Distribution	Negative Binomial		
Link Function	Log		
Dependent Variable	IF_pred	IF_pred	
Offset Variable	DY	DY	
Observations Used	348		
Missing Values	20		

Class Level Information

Class	Levels	Values
TYP	52	C1-1 C1-10 C1-11 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C1-9 C2-1 C2-10 C2-11 C2-12 C2-13 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C2-9 C3-1 C3-10 C3-11 C3-12 C3-2 C3-3 C3-4 C3-5 C3-6 C3-7 C3-8 C3-9 C4-1 C4-10 C4-11 C4-12 C4-2 C4-3 C4-4 C4-5 C4-6 C4-7 C4-8 C4-9 ...
Treatment	2	C T
TG	4	1 2 3 4
Construction	3	A N Y
CNTY	6	DEF(1) FUL(2) LIC(5) MUS(5) RIC(3) WAS(10)
MCL	3	1 2 4
SYS_CL	3	A L M
ACS	1	N
FC	4	2 6 7 8
AC	1	0
Lns	1	2

Parameter Information

Parameter	Effect
Prm1	Intercept
Prm2	lnADT
Prm3	lnTrcks

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	345	204.2954	0.5922
Scaled Deviance	345	204.2954	0.5922
Pearson Chi-Square	345	319.7588	0.9268
Scaled Pearson X2	345	319.7588	0.9268
Log Likelihood		-180.3850	

Analysis Results for Flatten Vertical Curve

The SAS System

14:55 Friday, July 2, 2004 2

The GENMOD Procedure

Algorithm converged.

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald 95% Confidence Limits		Chi-Square	Pr > ChiSq
Intercept	1	-4.3707	1.0860	-6.4993	-2.2421	16.20	<.0001
lnADT	1	0.7448	0.1459	0.4588	1.0309	26.05	<.0001
lnTrcks	1	-1.4310	0.2613	-1.9431	-0.9188	29.99	<.0001
Dispersion	1	1.2883	0.4563	0.6435	2.5791		

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (52 levels)
Number of Clusters	52
Clusters With Missing Values	4
Correlation Matrix Dimension	8
Maximum Cluster Size	7
Minimum Cluster Size	3

Algorithm converged.

Analysis Of GEE Parameter Estimates Empirical Standard Error Estimates

Parameter	Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
Intercept	-4.3707	1.2267	-6.7751	-1.9663	-3.56	0.0004
lnADT	0.7448	0.2388	0.2769	1.2128	3.12	0.0018
lnTrcks	-1.4310	0.4721	-2.3562	-0.5057	-3.03	0.0024

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
lnADT	1	9.73	0.0018
lnTrcks	1	9.19	0.0024

Analysis Results for Flatten Vertical Curve

Empirical Bayes (EB) Calculations for Injury and Fatality Crashes

TYP	Duration	Construction	ADT	ptotal	Cy	I_F	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)
T1-1	365	N	1693	0.200	1,000	0	0.251	0.796	0.159	0.032	0.218	0.014
T1-1	366	N	1805	0.239	1,196	1	0.312	0.765	0.418	0.098		
T1-1	365	N	1918	0.279	1,396	0	0.379	0.736	0.205	0.054		
T1-1	212	Y	1918	0.183	0,918	0	0.226	0.809	0.148	0.028		
T1-1	153	A	2030	0.183	0,918	0	0.226	0.809	0.148	0.028	0.200	0.012
T1-1	365	A	2145	0.347	1,737	0	0.501	0.691	0.240	0.074	0.378	0.043
T1-1	366	A	2260	0.366	1,835	1	0.539	0.679	0.569	0.182	0.399	0.048
T1-1	365	A	2160	0.361	1,808	2	0.529	0.683	0.881	0.280	0.393	0.047
T2-1	365	N	3935	0.632	1,000	2	1.147	0.551	1.246	0.559	1.207	0.175
T2-1	366	N	4360	0.686	1,086	3	1.293	0.531	1.772	0.831		
T2-1	365	N	4720	0.705	1,115	1	1.345	0.524	0.845	0.402		
T2-1	61	Y	4720	0.306	0,485	0	0.427	0.717	0.220	0.062		
T2-1	304	A	5080	0.619	0,980	2	1.113	0.556	1.232	0.547	1.182	0.168
T2-1	365	A	5440	0.753	1,191	5	1.482	0.508	2.844	1.400	1.437	0.248
T2-1	366	A	5800	0.778	1,232	3	1.559	0.499	1.891	0.947	1.486	0.265
T2-1	365	A	6160	0.803	1,270	6	1.633	0.492	3.445	1.752	1.533	0.282
T3-1	366	N	1300	0.798	1,000	1	1.619	0.493	0.900	0.456	0.613	0.113
T3-1	365	N	1220	0.694	0,869	0	1.314	0.528	0.366	0.173		
T3-1	365	N	1140	0.599	0,751	0	1.062	0.564	0.338	0.147		
T3-1	31	Y	1060	0.205	0,257	0	0.259	0.791	0.162	0.034	0.304	0.028
T3-1	334	A	980	0.396	0,496	0	0.598	0.662	0.262	0.089	0.332	0.033
T3-1	366	A	980	0.432	0,542	0	0.673	0.642	0.278	0.099	0.338	0.034
T3-1	365	A	990	0.441	0,552	0	0.691	0.638	0.281	0.102	0.346	0.036
T3-1	365	A	1000	0.450	0,564	0	0.712	0.633	0.285	0.105		

Analysis Results for Flatten Vertical Curve

TYP	Duration	Construction	ADT	ptotal	Cy	I_F	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)	
T4-1	366	N	7830	1.182	1.000	3	2.982	0.396	2.279	1.376	3.232	0.640	
T4-1	365	N	7710	1.240	1.049	6	3.221	0.385	4.168	2.563			
T4-1	365	N	7712	1.232	1.042	5	3.188	0.387	3.544	2.174			
T4-1	275	Y	7713	0.957	0.809	0	2.135	0.448	0.428	0.237			
T4-1	90	A	7715	0.573	0.484	0	0.995	0.576	0.329	0.140	1.565	0.150	
T4-1	366	A	7715	1.220	1.032	2	3.136	0.389	1.696	1.037	3.334	0.681	
T4-1	365	A	7717	1.204	1.019	2	3.074	0.392	1.688	1.027	3.293	0.664	
T4-1	365	A	7718	1.201	1.016	0	3.058	0.393	0.471	0.286	3.283	0.660	
								SE					
	5265	A			Actual	23	23.000	4.796	16.541		19.804	3.401	1.844
									1.147	ThetaHat	1.161		
	4384	N			Dispersic	1.2883			16.240	bias	1.009	Variance	Std Error
									1.352	ThetaHatU	1.151	0.068	0.261
									0.848				
Summary of CRF Calculations for Injury and Fatality Crashes								All Treatment Sites	CRF	-0.151		0.261	
									Z	-0.581			
									P-value	0.719			
								SE					
						7	7	2.646	16.907		14.165	2.437	
									1.147	ThetaHat	0.494		
									45.771	bias	1.012	Variance	Std Error
									1.352	ThetaHatU	0.488	0.036	0.190
									0.848				
								All Treatment Sites Except for T2	CRF	0.512		0.190	
									Z	2.695			
									P-value	0.004			

Analysis Results for Provide Interchange Lighting

APPENDIX G - EB Modeling Steps: Provide Interchange Lighting

Analysis Results for Provide Interchange Lighting

The SAS System

15:24 Friday, June 10, 2005 26

The GENMOD Procedure

Model Information

Data Set	WORK.DATA13		
Distribution	Negative Binomial		
Link Function	Log		
Dependent Variable	Ttl_f_Pred	Ttl_f_Pred	
Offset Variable	DY	DY	
Observations Used	193		

Class Level Information

Class	Levels	Values
TYP	30	C1-1 C1-10 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C1-9 C2-1 C2-10 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C2-9 C5-1 C5-2 C5-3 C5-4 C5-5 C5-6 C5-7 T1-1 T2-1 T5-1
Treatment	2	C T
Construction	1	N
CNTY	8	CAR(11) COL (11) JEF(11) LIC(5) LUC(2) MOT(7) POR(4) TRU(4)
MCL	3	1 2 4
SYS_CL	2	A M
ACS	2	L N
FC	5	2 6 7 14 16
AC	6	0 38 44 167 349 765

Parameter Information

Parameter	Effect
Prm1	Intercept
Prm2	lnADT
Prm3	SW

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	190	198.7546	1.0461
Scaled Deviance	190	198.7546	1.0461
Pearson Chi-Square	190	208.2414	1.0960
Scaled Pearson X2	190	208.2414	1.0960
Log Likelihood		-91.0480	

Algorithm converged.

The SAS System

15:24 Friday, June 10, 2005 27

The GENMOD Procedure

Analysis Results for Provide Interchange Lighting

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald 95% Confidence Limits		Chi-Square	Pr > ChiSq
Intercept	1	1.9432	1.7208	-1.4296	5.3160	1.28	0.2588
lnADT	1	-0.2212	0.2032	-0.6196	0.1771	1.18	0.2764
SW	1	0.0405	0.0168	0.0076	0.0734	5.81	0.0160
Dispersion	1	1.0381	0.1924	0.7219	1.4926		

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (30 levels)
Number of Clusters	30
Correlation Matrix Dimension	8
Maximum Cluster Size	8
Minimum Cluster Size	2

Algorithm converged.

Analysis Of GEE Parameter Estimates Empirical Standard Error Estimates

Parameter	Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
Intercept	1.9432	2.0982	-2.1691	6.0556	0.93	0.3544
lnADT	-0.2212	0.2564	-0.7237	0.2813	-0.86	0.3883
SW	0.0405	0.0317	-0.0216	0.1025	1.28	0.2012

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
lnADT	1	0.74	0.3883
SW	1	1.63	0.2012

Analysis Results for Provide Interchange Lighting

Empirical Bayes (EB) Calculations for Total Crashes

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)		
T1-1	364	N	6287	1.186	1.000	3	2.648	0.448	2.187	1.207	1.359	0.375		
T1-1	364	N	6250	1.188	1.001	0	2.653	0.448	0.532	0.294				
T1-1	183	Y	6195	0.596	0.503	0	0.966	0.618	0.368	0.141				
T1-1	181	A	6197	0.640	0.539	0	1.064	0.601	0.384	0.153	0.732	0.109		
T1-1	365	A	6160	1.292	1.089	0	3.023	0.427	0.552	0.316	1.479	0.444		
T1-1	364	A	6141	1.289	1.086	1	3.013	0.428	1.124	0.643	1.476	0.442		
T1-1	364	A	6105	1.291	1.088	0	3.020	0.427	0.552	0.316	1.478	0.443		
T2-1	366	N	8173	1.823	1.000	4	5.275	0.346	3.248	2.125	2.916	0.952		
T2-1	365	N	7900	1.832	1.005	3	5.316	0.345	2.598	1.702				
T2-1	365	Y	7650	1.845	1.012	2	5.379	0.343	1.947	1.279				
T2-1	303	Y	7354	1.545	0.847	1	4.024	0.384	1.209	0.745				
T2-1	62	A	7353	0.372	0.204	0	0.515	0.722	0.268	0.075	0.594	0.040		
T2-1	366	A	7041	2.216	1.215	0	7.312	0.303	0.671	0.468	3.543	1.406		
T2-1	365	A	6765	2.229	1.223	5	7.388	0.302	4.164	2.907	3.564	1.423		
T2-1	365	A	6470	2.251	1.235	1	7.512	0.300	1.375	0.963	3.600	1.452		
T5-1	366	N	7156	1.732	1.000	3	4.845	0.357	2.547	1.636	1.312	0.292		
T5-1	365	N	8978	1.643	0.948	0	4.443	0.370	0.607	0.383				
T5-1	365	N	10780	1.577	0.911	0	4.160	0.379	0.598	0.371				
T5-1	365	Y	12582	1.524	0.880	2	3.937	0.387	1.816	1.113				
T5-1	305	Y	14242	1.239	0.716	1	2.834	0.437	1.105	0.622				
T5-1	61	A	14379	0.247	0.143	0	0.311	0.796	0.197	0.040	0.187	0.006		
T5-1	365	A	16186	1.442	0.833	1	3.600	0.401	1.177	0.706	1.092	0.203		
T5-1	365	A	17988	1.408	0.813	1	3.468	0.406	1.166	0.692	1.067	0.193		
T5-1	365	A	19790	1.379	0.796	1	3.353	0.411	1.156	0.681	1.045	0.185		
								SE						
						Actual	10	10.000	3.162	12.785		19.859	6.347	2.519
						Dispersion 1.0381				1.301	theta	0.504		
										12.316	bias	1.016	Variance	Std Error
										1.759	Unbiased	0.496	0.028	0.166
Summary of CRF Calculations for Total Crashes										CRF	0.504	0.166		
										Z	3.035			
										P-value	0.0012			

Analysis Results for Provide Interchange Lighting

The SAS System 16:59 Tuesday, June 29, 2004 1

The GENMOD Procedure

Model Information

Data Set	WORK.DATA1
Distribution	Negative Binomial
Link Function	Log
Dependent Variable	Total
Offset Variable	DY
Observations Used	193

Class Level Information

Class	Levels	Values
TYP	30	C1-1 C1-10 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C1-9 C2-1 C2-10 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C2-9 C5-1 C5-2 C5-3 C5-4 C5-5 C5-6 C5-7 T1-1 T2-1 T5-1
Treatment	2	C T
Construction	1	N
CNTY	8	CAR(11) COL (11) JEF(11) LIC(5) LUC(2) MOT(7) POR(4) TRU(4)
MCL	6	1 2 3 4 5 6
SYS_CL	2	A M
ACS	2	L N
FC	5	2 6 7 14 16
AC	6	0 38 44 167 349 765
TG	3	1 2 5

Parameter Information

Parameter	Effect
Prm1	Intercept
Prm2	lnADT

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	191	152.3009	0.7974
Scaled Deviance	191	152.3009	0.7974
Pearson Chi-Square	191	185.1195	0.9692
Scaled Pearson X2	191	185.1195	0.9692
Log Likelihood		-151.1914	

Algorithm converged.

The SAS System 16:59 Tuesday, June 29, 2004 2

Analysis Results for Provide Interchange Lighting

The GENMOD Procedure

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald 95% Confidence Limits		Chi-Square	Pr > ChiSq
Intercept	1	-2.1013	2.2488	-6.5089	2.3062	0.87	0.3501
lnADT	1	0.1531	0.2485	-0.3340	0.6402	0.38	0.5379
Dispersion	1	1.3324	0.4750	0.6625	2.6796		

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (30 levels)
Number of Clusters	30
Correlation Matrix Dimension	8
Maximum Cluster Size	8
Minimum Cluster Size	2

Algorithm converged.

Analysis Of GEE Parameter Estimates Empirical Standard Error Estimates

Parameter	Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
Intercept	-2.1013	2.0466	-6.1126	1.9099	-1.03	0.3045
lnADT	0.1531	0.2319	-0.3015	0.6077	0.66	0.5092

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
lnADT	1	0.44	0.5092

Analysis Results for Provide Interchange Lighting

Empirical Bayes (EB) Calculations for Injury and Fatality Crashes

TYP	Duration	Construction	ADT	ptotal	Cy	Total	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)		
T1-1	364	N	6287	0.466	1.000	0	0.756	0.617	0.288	0.110	0.288	0.055		
T1-1	364	N	6250	0.466	0.999	0	0.755	0.617	0.287	0.110				
T1-1	183	Y	6195	0.233	0.500	0	0.306	0.763	0.178	0.042				
T1-1	181	A	6197	0.231	0.495	0	0.302	0.765	0.177	0.042	0.142	0.014		
T1-1	365	A	6160	0.465	0.997	0	0.753	0.617	0.287	0.110	0.287	0.055		
T1-1	364	A	6141	0.464	0.994	1	0.750	0.618	0.668	0.255	0.286	0.054		
T1-1	364	A	6105	0.463	0.993	0	0.749	0.618	0.286	0.109	0.286	0.054		
T2-1	366	N	8173	0.487	1.000	3	0.803	0.606	1.476	0.581	0.888	0.175		
T2-1	365	N	7900	0.483	0.992	0	0.794	0.608	0.294	0.115				
T2-1	365	Y	7650	0.481	0.987	1	0.789	0.610	0.684	0.267				
T2-1	303	Y	7354	0.397	0.815	0	0.606	0.654	0.260	0.090				
T2-1	62	A	7353	0.081	0.167	0	0.090	0.902	0.073	0.007	0.148	0.005		
T2-1	366	A	7041	0.476	0.977	0	0.778	0.612	0.291	0.113	0.868	0.168		
T2-1	365	A	6765	0.472	0.969	2	0.768	0.614	1.062	0.410	0.861	0.165		
T2-1	365	A	6470	0.469	0.962	1	0.761	0.616	0.673	0.259	0.855	0.162		
T5-1	366	N	7156	0.477	1.000	1	0.781	0.611	0.680	0.264	0.414	0.053		
T5-1	365	N	8978	0.493	1.032	0	0.816	0.604	0.297	0.118				
T5-1	365	N	10780	0.507	1.062	0	0.849	0.597	0.302	0.122				
T5-1	365	Y	12582	0.519	1.087	1	0.877	0.591	0.715	0.292				
T5-1	305	Y	14242	0.442	0.926	1	0.702	0.629	0.649	0.240				
T5-1	61	A	14379	0.088	0.185	0	0.099	0.895	0.079	0.008	0.077	0.002		
T5-1	365	A	16186	0.539	1.130	0	0.927	0.582	0.314	0.131	0.468	0.067		
T5-1	365	A	17988	0.548	1.148	0	0.948	0.578	0.317	0.134	0.475	0.069		
T5-1	365	A	19790	0.556	1.165	0	0.968	0.574	0.319	0.136	0.482	0.072		
SE														
			3588	Actual		4	4.000	2.000	4.547			5.235	0.887	0.942
			2555	Dispersion		1.3324		0.463		theta	0.764			
							3.625		bias	1.032		Variance	Std Error	
							0.518		Unbiased	0.740		0.145	0.381	
Summary of CRF Calculations for Injury/Fatal Crashes										CRF	0.260	0.381		
										Z	0.682			
										P	0.248			

Analysis Results for Provide Interchange Lighting

The SAS System

15:02 Friday, May 27, 2005 92

The GENMOD Procedure

Model Information

Data Set	SASUSER.HLNIGHT	
Distribution	Negative Binomial	
Link Function	Log	
Dependent Variable	Ttl_f_Pred	Ttl_f_Pred
Offset Variable	DY	DY
Observations Used	193	
Missing Values	17	

Class Level Information

Class	Levels	Values
TYP	30	C1-1 C1-10 C1-2 C1-3 C1-4 C1-5 C1-6 C1-7 C1-8 C2-1 C2-10 C2-2 C2-3 C2-4 C2-5 C2-6 C2-7 C2-8 C5-1 C5-2 C5-3 C5-4 C5-5 C5-6 C5-7 T1-1 T2-1 T5-
TG	3	1 2 5
Treatment	2	C T
Construction	3	A N Y
CNTY	8	CAR(11) COL (11) JEF(11) LIC(5) LUC(2) MOT(7) POR(4) TRU(4)
MCL	3	1 2 4
SYS_CL	2	A M
ACS	2	L N
FC	5	2 6 7 14 16
AC	6	0 38 44 167 349 765
Lns	2	2 4

Parameter Information

Parameter	Effect
Prm1	Intercept
Prm2	lnNDT
Prm3	lnNtrcks

The SAS System

15:02 Friday, May 27, 2005 93

The GENMOD Procedure

Parameter Information

Parameter	Effect
Prm4	SW

Analysis Results for Provide Interchange Lighting

Criteria For Assessing Goodness Of Fit

Criterion	DF	Value	Value/DF
Deviance	189	154.4781	0.8173
Scaled Deviance	189	154.4781	0.8173
Pearson Chi-Square	189	207.1240	1.0959
Scaled Pearson X2	189	207.1240	1.0959
Log Likelihood		-141.8275	

Algorithm converged.

Analysis Of Initial Parameter Estimates

Parameter	DF	Estimate	Standard Error	Wald 95% Confidence Limits		Chi-Square	Pr > ChiSq
Intercept	1	6.6802	2.2612	2.2483	11.1122	8.73	0.0031
lnNDT	1	-1.4183	0.3595	-2.1229	-0.7137	15.56	<.0001
lnNtrcks	1	1.1301	0.3486	0.4469	1.8134	10.51	0.0012
SW	1	0.0804	0.0294	0.0228	0.1380	7.49	0.0062
Dispersion	1	0.6431	0.3124	0.2482	1.6661		

NOTE: The negative binomial dispersion parameter was estimated by maximum likelihood.

GEE Model Information

Correlation Structure	Independent
Subject Effect	TYP (30 levels)
Number of Clusters	30
Clusters With Missing Values	3
Correlation Matrix Dimension	9
Maximum Cluster Size	8
Minimum Cluster Size	2

Algorithm converged.

Analysis Of GEE Parameter Estimates Empirical Standard Error Estimates

Parameter	Estimate	Standard Error	95% Confidence Limits		Z	Pr > Z
Intercept	6.6802	1.8703	3.0145	10.3460	3.57	0.0004
lnNDT	-1.4183	0.3346	-2.0740	-0.7625	-4.24	<.0001
lnNtrcks	1.1301	0.3726	0.3999	1.8603	3.03	0.0024
SW	0.0804	0.0374	0.0071	0.1538	2.15	0.0317

Analysis Results for Provide Interchange Lighting

Wald Statistics For Type 3 GEE Analysis

Source	DF	Chi-Square	Pr > ChiSq
lnNDT	1	17.97	<.0001
lnNtrcks	1	9.20	0.0024
SW	1	4.62	0.0317

Analysis Results for Provide Interchange Lighting

Empirical Bayes (EB) Calculations for All Nighttime Crashes

TYP	Duration	DY	Construction	NADT	total	Cy	Total	V(count)	Alpha	EB lambda	V(EB)	Projected Count	V(PC)
T1-1	365	0	N	1666	0.10	1.00	2	0.106	0.94	0.21	0.013	0.15	0.005
T1-1	365	0.00	N	1656	0.10	1.01	0	0.107	0.94	0.09	0.006		
T1-1	183	-0.69	Y	1646	0.05	0.51	0	0.052	0.97	0.05	0.002		
T1-1	181	-0.70	A	1591	0.24	2.43	0	0.279	0.87	0.21	0.028	0.37	0.027
T1-1	365	0.00	A	1581	0.49	4.94	0	0.647	0.76	0.37	0.090	0.76	0.112
T1-1	364	0.00	A	1576	0.49	4.95	0	0.648	0.76	0.37	0.090	0.76	0.113
T1-1	364	0.00	A	1567	0.50	4.99	0	0.655	0.76	0.38	0.091	0.77	0.115
T2-1	366	0.00	N	2171	0.18	1.00	0	0.201	0.90	0.16	0.017	0.21	0.011
T2-1	365	0.00	N	2099	0.19	1.05	1	0.211	0.89	0.28	0.030		
T2-1	365	0.00	Y	2033	0.20	1.10	0	0.222	0.89	0.17	0.020		
T2-1	303	-0.19	Y	1954	0.17	0.96	0	0.192	0.90	0.16	0.016		
T2-1	62	-1.77	A	2076	0.53	2.93	0	0.704	0.75	0.39	0.099	0.62	0.095
T2-1	366	0.00	A	1988	3.30	18.37	0	10.318	0.32	1.06	0.719	3.92	3.744
T2-1	365	0.00	A	1910	3.49	19.39	1	11.300	0.31	1.77	1.222	4.14	4.171
T2-1	365	0.00	A	1826	3.71	20.65	0	12.581	0.30	1.10	0.773	4.41	4.733
T5-1	366	0.00	N	1916	0.46	1.00	1	0.595	0.77	0.58	0.133	0.47	0.041
T5-1	365	0.00	N	2404	0.33	0.72	0	0.403	0.82	0.27	0.048		
T5-1	365	0.00	N	2887	0.26	0.56	0	0.298	0.86	0.22	0.031		
T5-1	365	0.00	Y	3369	0.21	0.45	0	0.233	0.88	0.18	0.021		
T5-1	305	-0.18	Y	3814	0.14	0.31	0	0.158	0.92	0.13	0.011		
T5-1	61	-1.79	A	3851	0.03	0.06	0	0.029	0.98	0.03	0.001	0.03	0.000
T5-1	365	0.00	A	4335	0.14	0.31	0	0.157	0.92	0.13	0.011	0.15	0.004
T5-1	365	0.00	A	4817	0.12	0.27	0	0.134	0.93	0.11	0.008	0.13	0.003
T5-1	365	0.00	A	5300	0.11	0.24	0	0.116	0.93	0.10	0.007	0.11	0.002
3588		9.83				Actual	1	1	1	SE		16.17	13.119
2557		7.01								6.02	ThetaHat	0.06	3.62
										0.61	bias	1.05	Variance
										1.82	unbiased	0.06	0.003
										0.26		0.06	Std Error
										2.36			0.06
										CRF		0.94	
										Z		16.37284	
										P-value		0	